PRELIMINARY STORMWATER POLLUTION PREVENTION PLAN

RIVER PARK CENTER PROJECT CITY OF YONKERS, NY

Prepared for:

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1.0 SCOPE OF REPORT

This Report has been prepared to provide the Preliminary Stormwater Pollution Prevention Plan (SWPPP) for the River Park Center Project (the Project) that is located in the City of Yonkers, Westchester County, New York. This Report addresses the requirements set forth in the New York State Department of Environmental Protection's (NYSDEC) Pollution Discharge Elimination System (SPDES) for discharges fro construction activities General Permit GP-02-01 (General Permit), and the City of Yonkers' Stormwater Management Program. The General Permit covers discharges that are associated with construction activity, specifically activities that result in the disturbance of one (1) acre or more of total land area.

To be covered under the General Permit, the project would be required to conform to the technical standards for stormwater quantity and quality controls presented in the New York State Stormwater Management Design Manual ("DEC Design Manual"). Since this is a redevelopment project, stormwater management and water quality for portions of the Project with existing impervious area will also be governed Chapter 9, "Redevelopment Projects" of the DEC Design Manual. The provision of stormwater management practices in redevelopment follow an approach to balance between 1) maximizing improvements in site design that can reduce the impacts of stormwater runoff, and 2) providing a maximum level of on-site treatment that is feasible given the redevelopment project site constraints. The primary focus of the chapter is to identify alternative practices and their sizing criteria.

As a Municipal Separate Stormwater System, the City has developed a Stormwater Management Program ("SWMP") as required for coverage under the General Permit. The SWMP includes a listing of the Best Management Practices ("BMPs") that will be implemented by the City to achieve the regulatory standard of reducing pollutants in the City's stormwater to the maximum extent possible. With respect to construction activities, the BMPs include implementation of education and training of construction site operators; implementation of site plan review that includes ensuring that sediment and erosion control are part of the construction process; conducting site inspections and enforcing compliance with the issued building permit, approved site plans and the City Erosion and Sediment Control Code. Compliance with the General Permit will meet the objectives of the City SWMP.

1.1 Responsibilities of the Participants

It is the responsibility of the Owner, General Contractor, and subcontractors to comply with the measures set forth in this SWPPP and implement pollutant control measures which retain surface water quality and prevent sediment laden runoff from entering rivers, streams, estuaries, wetlands and other sensitive environments. The following outlines the responsibilities of all participants.

1.1.1 Owner's Engineer

- 1. Prepare the SWPPP using good Engineering practices, best management practices and in compliance with the General Permit.
- 2. Prepare Notice of Intent (NOI) for the Owner for submission to the NYSDEC.
- 3. Provide copies of the SWPPP and the "Acknowledgement of Notice of Intent" to the local government agencies having jurisdiction or regulatory control over the project.
- 4. Review the site prior to the beginning of construction and certify in an inspection report that the appropriate pre-construction erosion and sediment control measures outlined herein and that are required by the General Permit have been installed and will operate as designed.
- 5. Conduct on-site inspections every 7 days and within 24 hours of 0.5 inches or greater of rainfall for general compliance with the SWPPP and the General Permit. Inspection reports will be provided to the Owner within 24 hours of the field inspection. Any problem areas of areas in need of additional stabilization shall be made clear to the Owner.
- 6. Review onsite Contractor's SWPPP records to ensure compliance and update them as required or necessary.
- 7. Update the SWPPP each time there is a significant modification to the design or construction that may have a significant effect on the potential for discharge of pollutants into receiving waters.

- 8. When construction is complete, provide the Owner with certification that an inspection has been completed verifying that the site has undergone final stabilization.
- 9. When the site has undergone final stabilization, prepare the Notice of Termination (NOT) for the Owner for submission to the NYSDEC.

1.1.2 Owner/Operator/Permittee

The following is a summary of the Owner's responsibilities:

- 1. Sign the NOI and certify the SWPPP by signing the Owner's Certification statement. Submit the NOI to NYSDEC.
- 2. When the Owner receives a letter of "Acknowledgement of Notice of Intent" from the NYSDEC, post a copy of this letter at the site in a prominent place for the public viewing.
- In accordance with the requirements of the General Permit, the Owner/Operator shall maintain a record of all inspection reports in a site logbook. The site logbook shall be maintained on site and be made available to the permitting authority upon request. The site logbook shall contain the following documents:
 - a. NYSDEC Notice of Intent
 - b. NYSDEC Notice of Acknowledgement
 - c. Stormwater Pollution Prevention Plan (SWPPP)
 - d. Owner/Operator SPDES Permit Certification (Signed copy)
 - e. Contractor/Subcontractor SPDES Permit Certification (Signed copy)
 - f. Pre Construction Site Assessment Report
 - g. Site Assessment Reports
 - h. Monthly Assessment Logs The Operator shall post at the site, in a publicly- accessible location, a summary of the site inspection activities on a monthly basis.
 - i. Quarterly Reports
 - j. Final Certification

- k. SWPPP Modifications
- I. SDPES General Permit GP-02-01 for Stormwater Discharges from Construction Activity
- 4. Ensure the SWPPP report, inspection reports and inspection quarterly summaries are certified by an authorized person who has responsibility for the overall operation of the site such as a project manager or site superintendent. Certification of these documents is executed by signing the certifying statements and at the end of the inspection reports.
- 5. Require the General Contractor and all Sub Contractors involved with construction activity that disturbs site to fully implement the SWPPP and the requirements set forth in the General Permit. The SWPPP should be certified by the General Contractor and all Sub Contractors involved with earth disturbance during construction by signing the certifying statement.
- 6. Upon project completion and when the site has reached final stabilization, the Owner should sign the Notice of Termination (NOT) prepared by the Owner's Engineer and submit to NYSDEC.
- 7. Retain all site records and documentation including Engineering reports, SWPPP reports, SWPPP inspection reports and all records of data used to complete the NOI for a minimum of 3 years from the date the site reached final stabilization.
- 8. Provide an Operation & Maintenance (O&M) manual to any future Owners.

1.1.3 Contractors and Sub-Contractors

The following is summary of the Contractor's responsibilities:

- 1. Implement fully the SWPPP and the requirements set forth in the SPDES General Permit. Certify the SWPPP by signing the Contractor's Certification statement.
- 2. Provide the names and addresses of all sub-contractors involved in construction activities that disturb site soils for inclusion in the SWPPP.

- 3. Ensure all sub-contractors involved in construction activities that disturb site soils to implement fully the SWPPP and the requirements set forth in the SPDES General Permit. All sub-contractors must certify the SWPPP by signing the Contractor's Certification statement contained in (Appendix A) of this report.
- 4. Conduct inspections on a regular basis of the erosion and sedimentation controls installed at the site. Maintain and repair as necessary all erosion and sedimentation controls.

2.0 SITE DESCRIPTION

This section briefly describes existing site conditions on and adjacent to the project site as they relate to Surface Water Management planning considerations. Subsequent sections contain a description of the manner in which site runoff will be managed to minimize impacts on areas adjacent to the site and the compliance with the referenced regulations.

2.1 Locations

For the purpose of the description of the stormwater infrastructure and the analysis of the existing and proposed stormwater runoff and impacts, the Project site and the surrounding area has been defined as four (4) study areas (See Drawing No. 1). The study areas include the following:

- <u>River Park Center</u> The area is bounded by Elm Street, Palisades Avenue, New Main Street and Nepperhan Avenue.
- <u>Government Center</u> This area is located at the northwest corner of the intersection of New Main Street and Nepperhan Avenue.
- The Cacace Center The area is bounded by South Broadway, New Main Street and Nepperhan Avenue. This area includes the existing parking lots north of the Cacace Justice Center that will be developed as part of this Project.
- Elm and Palisades Center The area is located at the northeast corner of Elm Street and Palisades Avenue. This area includes existing buildings, and parking areas behind the buildings, adjacent to the Saw Mill River.

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The existing stormwater runoff within the project area is collected by the City of Yonkers (the City) combined storm and sanitary collection system, the New York State Department of Transportation (NYSDOT) storm drain system within Nepperhan Avenue and direct runoff to the Saw Mill (Nepperhan) River. A description of the existing stormwater infrastructure in the project area is as follows:

River Park Center

The combined sewers are located within the surrounding streets as follows:

- o *Elm Street* Combined sewers from north and north east of Elm Street are conveyed south along Elm Street, toward Palisades Avenue, by 36-inch to 48-inch brick sewers. A 36-inch cast iron sewer is located beneath the Elm Street Bridge over the Saw Mill River.
- o John, James & School Street and Engine Place Parking Area Within this area, 8-inch to 18-inch combined sewers convey flow to the Palisades Avenue combined sewer. At the present time only the buildings on School Street and several inlets convey stormwater to these sewers. Prior buildings in the area of Engine Place were historically connected to these combined sewers. Currently, the runoff from the grass area between Palisades Avenue and Engine Place and the Engine Place parking lot are collected by inlets that flow to the Saw Mill River.
- o Palisades Avenue A 36-inch cast iron and 48-inch brick combined sewers are located along Palisades Avenue and flow toward Getty Square.
- o Nepperhan Avenue- The combined sewer in Nepperhan Avenue conveys flows from northeast. At the intersection of Elm Street, an 18-inch clay and a 24-inch by 36-inch brick sewer flows southwest toward New Main Street and conveys flows from the south and east of Nepperhan Avenue. A 12-inch and 18-inch clay sewer begins west of New Main Street, south of the City Hall Government Center. This sewer conveys flows from the Government Center and prior buildings south of Nepperhan Avenue.
- o New Main Street- 36-inch and 48-inch brick combined sewers convey the flow from Nepperhan Avenue and along New Main Street to Getty Square.

Within Getty Square a regulator allows the City trunk combined sewer to overflow into the Westchester County trunk sewer. The City combined sewers are tributary to the

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Westchester County Main Street Pump Station located at the west end of Main Street on the west side of the railroad. Another regulator is also located at the Main Street Pump Station site that overflows directly to the Hudson River during times of heavy rainfall. From the Getty Square regulator, the 72-inch County trunk sewer flows north to the North Yonkers County Pump Station located along the Hudson River. The North Yonkers Pump station also has an overflow to the Hudson. The overflow is chlorinated prior to discharge during times of heavy rain.

A separate storm drain system was installed with Nepperhan Avenue when the roadway was previously reconstructed by the NYSDOT. This storm drain system is generally located between New Main Street and Waverly Street and discharges directly to the Saw Mill River, southwest of the Henry Herz Street Bridge.

- Government Center (see Figure) The majority of this area drains to the combined storm sewers in New Main Street and Nepperhan Avenue. Some overland flow from the Government Center flows toward Nepperhan Avenue and into the NYSDOT storm drain system.
- The Cacace Center Runoff from the northeastern portion of this area flows toward Nepperhan Avenue and South Broadway. There is an 18-inch clay combined sewer in Broadway that flows north toward Getty Square. The eastern portion of this area, including the Robert C. Cacace Justice Center, flow to a series of 12-inch clay pipes toward the 40-inch brick combined sewer in New Main Street. This sewer flow north across Nepperhan Avenue and to Getty Square. Portions of the parking lot in the northern portion of the area flow toward Nepperhan Avenue and are picked up by the roadway drainage system.
- <u>Elm and Palisades Center- A</u> portion of the runoff from the buildings along Palisades Avenue is assumed to flow to the 15-inch clay combined sewer in Palisades Avenue. The balance of the site drains directly to the River.

2.2 Watercourses/Wetlands

The Saw Mill River flows through the River Park Center, from Elm Street to Palisades Avenue. The Saw Mill River, formerly known as the Nepperhan River, is a tributary of the Hudson River and is located within Westchester County, New York. The River's headwaters are located in the Town of New Castle and it flows south along the Saw Mill

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River Parkway to the discharge point in the City of Yonkers, NY. The drainage basin is approximately 23 miles long with an average width of 1.4 miles and an average slope of 22 feet/mile. The contributing watershed of approximately 26.5 square miles and encompasses portions of New Castle, North Tarrytown, Tarrytown, Pleasantville, Greenburgh, Mount Pleasant, Elmsford, Irvington, Dobbs Ferry, Ardsley, and Hastings-on Hudson.

Throughout the Project area, beginning at the Elm Street Bridge, the Saw Mill River has an open section that flows in a southwesterly direction, under the bridge at the John Street extension and enters a culvert at the School Street Bridge. The River proceeds under the Henry Herz parking facility in a westerly direction parallel to Nepperhan Avenue and reimmerges west of Henry Herz Street. The River then turns northward and entering the flume at Ann Street. The River falls approximately 13 feet in elevation along this length. The River bottom throughout this area is naturalized, with a cobble/boulder streambed. The open portions of the River have a channel width between 30 and 40 feet and steep sloping or vertical banks stabilized with concrete, stone riprap or masonry walls. The bridges over the River and the culverted sections are aligned with the river and have dimensions greater than or equal to the channel width. At Ann Street, the River enters the flume and flows in a northerly direction toward Getty Square.

In accordance with 6 NYCRR Part 859, Saw Mill River Drainage Basin, the River from the Hudson River mouth to 1,100 feet upstream is classified as Class SB saline surface water, which the best usages are primary and secondary contact recreation and fishing. These waters are also suitable for fish propagation and survival. Through the Project site to Saw Mill River is Class C, which are suitable for fish propagation and survival and for primary and secondary contact recreation.

2.3 Land Cover

The entire project area has been subject to prior development. Pervious areas are limited to lawn areas and limited areas of brush and trees along the Saw Mill River. Building, parking lots, roadways, sidewalks and gravel areas cover the balance of the area.

2.4 Soils

The USDA Natural Resources Conservation Service, National Cooperative Soil Survey indicates that there is only one type of soil present on the proposed site.

Table I below summarizes the characteristics of the soil present on the site and the respective areas.

Table 1
Soil Characteristics

Map Unit	Area (acres)	Soil Names	Water Table (ft)	Depth to Bedrock	Hydrologic Group
Uf (2)	Entire	Urban	5'-20' (1)	15' to greater	D
	Project	Lawn		than 70'	

Source:

- 1. Based on borings developed for project
- 2. Natural Resource Conservation Center Web Soil Survey

2.5 Floodplain

The site is located within the Saw Mill River drainage basin. A review of the Flood Insurance Rate Map for the City of Yonkers (Community Panel Number 360936-0010-C), last revised January 21, 1988 indicates that the site is located within Zone A, which indicates no base flooding is determined. A separate hydraulic analysis of the Saw Mill River was prepared for the Project to determine the existing 100-year flood elevation through the site.

2.6 Rainfall Data

Rainfall data utilized in the analysis was from the NYSDEC Design Manual. The data used specific to the Project site and various 24-hour storm events are presented in Table 2 below.

Table 2 Rainfall Data

24-Hour Storm Event	Type III, 24-Hour Rainfall (inches)
Water Quality, Rainfall	1.3
2-Year	3.5
10-Year	5.0
25-Year	6.0
100-Year	7.5

3.0 METHODOLOGY

3.1 Stormwater Management

The Stormwater Management (SWM) Plan has been designed in accordance with Appendix D of the General Permit and the following publications:

- "Urban Hydrology for Small Watershed" (Technical Release No. 55), published by the United States Department of Agriculture, Soil Conservation Service, dated June 1986.
- New York State Stormwater Management Design Manual, August 2003.

The pre and post-development runoff rates provided in this Report were calculated using the computer software program entitled "WinTR-55" published by USDA National Resources Conservation Service. This program incorporates the methodology used in SCS TR-20 and TR-55 to compute flood hydrographs.

3.2 Water Quality

The Water Quality (WQ) management measures and designs described herein are in accordance with the pertinent portions of Chapters 4 through 7 of the NYSDEC Design Manual, as required in Part III.D.1 of the General Permit. The objective of a WQ management system is to meet the pollutant removal goals by capturing and treating 90% of the average annual stormwater runoff volume, otherwise known as the Water Quality Volume (WQv). By meeting the WQv requirements through employment of acceptable Stormwater Management Practices (SMP) listed in the NYSDEC Design Manual, the project will, by default, meet water quality objectives.

Acceptable SMP for water quality treatment must meet the following criteria:

- Capture and teat the full water quality volume (WQ_v).
- 80% TSS removal and 40% TP removal.
- Acceptable longevity in the field.
- Incorporate a pretreatment mechanism.

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3.3 Channel Protection Volume

Stream Channel Protection Volume Requirements (Cpv), as stated in Section 4.3 of the NYSDEC Design Manual, are designed to protect stream channels from erosion. This goal is accomplished by providing 24-hour extended detention of the one (1)-year, 24-hour storm event. However, Cpv is not required if discharge is to a tidal water or a fourth order stream. The Saw Mill River in Yonkers is greater than a fourth order stream and the project will not require Channel Protection Volume. In addition, the analysis of the impact of runoff on the Saw Mill River floodplain has indicated that there will be no downstream impact.

4.0 HYDROLOGIC AND HYDRAULIC ANALYSIS

4.1 Existing Conditions

A detailed investigation of the existing site and surrounding area was undertaken to allow an understanding of the surface runoff patterns on, and adjacent to the project site. Following a review of existing topography and site conditions four (4) separate study areas were defined for the Project site and surrounding area for the stormwater management analysis. The study areas include the following:

- River Park Center This entire area has been subject to prior development and disturbance. Currently it is comprised of approximately 9.69 acres of buildings, roads and parking, 3.95 acres of lawn, grass and the wooded and brush area adjacent to the River and 0.36 acres the existing river watercourse. Stormwater runoff from approximately 4.36 acres of this area, (designated Drainage Area A-3), currently drains directly to the Saw Mill River by overland flow (See Drawing No.4). Approximately 6.68 acres drain to storm drain systems that outlet to the Saw Mill River, designated Drainage Area A-2 (See Drawing No.3). Historically, the majority of this area discharged to the combined sewers. However, after demolition of prior buildings most of the area from Palisades Avenue to John Street now goes to the River. The remaining 2.96 acres of this area discharges to the City of Yonkers combined sewer system, designated Drainage Area A-1 (See Drawing No.2).
- Government Center This area, designated Drainage Area B, is comprised of approximately 1.79 acres of buildings, road and parking and 1.03 acres of lawn and grass (See Drawing No.5). Stormwater runoff from small portions of this site flow

toward the Nepperhan Avenue and the balance of the runoff flows toward New Main Street where it is collected by the City of Yonkers combined sewer system. However, for the runoff calculations, all flows are assumed to be directed to the combined sewers.

- The Cacace Center This entire area has been subject to prior development and disturbance. Currently it is comprised of approximately 2.55 acres of buildings roads and parking of which 2.38 acres is lawn and grass. This area has been divided into two separate drainage areas for the analysis. The western portion, designated Drainage Area C-2, flows to the intersection of South Broadway and Nepperhan Avenue where it enters the City of Yonkers combined sewer system. The eastern portion, designated Drainage Area C-1, flows to the intersection of New Main Street and Nepperhan Avenue, where portions of the overland flow enters the Nepperhan Avenue storm drain system that discharges to the Saw Mill River (See Drawing No.5).
- <u>Elm and Palisades Center</u> This entire area has been subject to prior development and disturbance and is comprised of approximately 0.67 acres of buildings and parking areas and approximately 0.12 acres of trees and brush along the river. Approximately 0.64 acres of this area flows to the rear (east) portion of the site and directly into the Saw Mill River, Designated Drainage Area D-2 (See Drawing No.6).

Design Points were identified for each of the existing drainage areas. The Design Points represent the location where the majority of runoff from the respective drainage area exits the site. The same design points are also identified in post-development conditions so that a comparison can be made between the pre- and post-development conditions. The design points are shown on the above referenced drawings. A description of each of the design points follows:

- (1) Design Point A For the purpose of determining existing peak flow rates, six (6) design points were used for the analysis. The peak flows from each Design Point was combined to determine the total flow to the Hudson River and the total flow from the River Park Center area.
- (2) Design Point B The combined runoff from the existing Government Center Parking Garage, building and landscape area.
- (3) Design Point C Drainage Area C is divided into two (2) sub areas with separate Design Points. Design Point C-1 has been established for the runoff toward New

Main Street and Design Point C-2 for the runoff toward the combined sewer in Broadway.

(4) Design Point D –Runoff from the entire Palisades Avenue Center site flows toward the combined sewer in Palisade Avenue, designated Design Point D-1.

A summary of the existing peak discharge rates from the Project Site is shown in Table 3.

Table 3
Table of Existing Peak Stormwater Discharge Rates

Location Design Year Storm (CFS)					
Location	2-Year	10-Year	25-Year	100-Year	
Rive	er Park Center (D	rainage Area A)			
Design Point A-2A	14.0	21.5	26.5	33.9	
Design Point A- 3A	8.7	13.8	17.2	22.2	
Design Point A-3B	1.4	2.2	2.7	3.5	
Sub-total to Saw Mill River (1)	22.0	34.3	42.4	54.5	
Design Point A-1A	5.2	7.7	9.3	11.7	
Design Point A-1B	2.4	3.5	4.2	5.2	
Design Point A-1C	1.4	2.1	2.5	3.1	
Sub-total to Combined Sewer (1)	9.0	13.2	16.0	20.1	
Total Drainage Area A (1)	30.7	42.3	58.2	74.6	
Gove	ernment Center (Drainage Area B)			
• Design Point B-1 (to Combined Sewer)	6.3	9.7	11.9	15.2	
• Design Point B-2 (to Saw Mill River)	0.7	1.2	1.5	1.9	
Total Drainage Area B (1)	7.1	10.9	13.4	17.2	
Ca	cace Center (Dra	ainage Area C)			
Design Point C-1 (toward Nepperhan Ave/River)	6.1	9.6	11.9	15.4	
Design Pont C-2 (to combined sewer)	5.6	8.8	10.9	14.0	
Total Drainage Area C (1)	11.7	18.4	22.8	29.4	
Palis	ades Avenue Cen	ter (Drainage D)			
Design Point D-1 (to Combined Sewer)	2.4	3.6	4.3	5.5	
River Park Center, Government	Center, Cacace	Center and Palisa	des Avenue Cente	r Totals	
To Saw Mill River (1)	28.6	44.9	55.7	71.7	
To Combined Sewers (1)	23.4	35.2	43.1	54.8	

⁽¹⁾ Total Discharge is based on sum of hydrographs

⁽²⁾ Portions of the Government Center drain toward Nepperhan Avenue/River. For this analysis, for existing conditions, the entire area is assumed to flow to the combined sewer.

4.2 Proposed Conditions

4.2.1 Proposed Condition Stormwater Runoff

Based on the building program for the Project, an analysis of the proposed stormwater runoff conditions was performed to determine the impact of the project on the existing City combined sewer system and to the Saw Mill River. The analysis utilized the same four (4) study areas as used for the existing condition analysis.

To the extent possible, the proposed drainage from the developed area will be conveyed by new storm drains and discharged to the Saw Mill River to reduce the runoff to the City combined sewer system. A description of the impact of the development on the stormwater runoff from each of the study areas is as follows:

- River Park Center The runoff from the proposed development will be directed to discharge directly to the Saw Mill River. The only portions of the site that will continue to discharge to the City's combined sewers will be the property on the southeast corner of New Main Street and Palisade Avenue (designated Drainage Area A-1A) and the Church property along Nepperhan Avenue (designated Drainage Area A-1C), which are not part of the Project. The area of runoff to the combined sewers is reduced from 3.2 acres to approximately 0.71 acres. The building and parking garages will be conveyed to outlets along the Riverwalk at the site. Runoff from the pedestrian areas along the Saw Mill River will drain directly to area drains and then to the River. All discharges will pass through water quality structures. The River Park Center development (designated Drainage Area A-1A) includes the turf baseball stadium on the roof of the building. This will encompass approximately 3 acres of turf area. With the construction of the stadium, the amount of impervious area within this portion of the Project will not increase over existing conditions.
- Government Center This area (designated Drainage Area B) will include the new parking garage. Runoff form the entire garage will be conveyed in a new storm drain and outlet to the Saw Mill River. The construction of the garage will result in an increase of approximately 0.9 acres of impervious area. The runoff from the garage will pass through an underground filtering device prior to discharge to the River.
- <u>The Cacace Center</u> The construction in this area will be comprised of the parking garage, Fire Department building and the office building/hotel. In order to not impact

the peak stormwater discharge toward the Broadway combined sewer, runoff from the entire garage structure will be directed to the Nepperhan Avenue and New Main Street storm drains and discharge to the Saw Mill River. As shown in Table III. D-2 below, the stormwater runoff to the combined sewer will be reduced.

Table III. D -2 Comparison of Peak Runoff to Broadway Combined Sewer from Cacace Center

Drainage Area C-2 Discharge to	Design Year Storm (CFS)					
Broadway Combined Sewer	2-Year	10-Year	25-Year	100-Year		
Existing Condition	5.6	8.8	10.9	14.0		
Developed Condition	4.1	6.6	8.2	10.6		
Difference	-1.5	-2.2	-2.7	-3.4		

The total impervious area from the Cacace Center developed area will increase by approximately 0.5 acres. Runoff will be directed to water quality and underground-filtering structures prior to discharge enters the Nepperhan Avenue storm drain system and discharged to the Saw Mill River.

 <u>Palisades Avenue Center</u> - This entire area will discharge to the proposed storm drain system in Palisades Avenue. See Section III-H 3.b for further discussion of separation of combined sewers. A summary of the post-development condition peak stormwater peak discharge rates from each of the sites (except Palisades Point) is provided in Table III. D-3.

Table III. D -3
Post Development Peak Stormwater Discharge Rates

Location	Design Year Storm (CFS)				
Location	2-Year	10-Year	25-Year	100-Year	
Rive	er Park Center (D	rainage Area A)			
Design Point A-2A (to Saw Mill River)	36.5	54.1	65.7	83.0	
 Design Point A-1A (to combined sewer) 	0.9	1.3	1.5	1.9	
 Design Point A-1C (to combined sewer) 	1.2	1.7	2.1	2.6	
Sub-total to Combined Sewer (1)	2.1	./0	3.6	4/5	
Total Drainage Area A (1)	38.6	57.1	69.3	87.6	
Gove	ernment Center (I	Orainage Area B)			
Design Point B (to Saw Mill River)	8.1	11.8	14.2	17.8	
Ca	acace Center (Dra	ninage Area C)			
Design Point C-1 (to Saw Mill River)	9.1	13.1	15.7	19.7	
 Design Pont C-2 (to combined sewer) 	4.1	6.6	8.2	10.7	
Total Drainage Area C (1)	13.2	19.7	24.0	30.4	
Palisade Avenue Office Building					
Design Pont D1 (to River)	2.4	3.6	4.3	5.5	
River Park Center, Government Ce	nter, Cacace Cen	ter and Palisade A	venue Office Buil	ding Totals	
To Saw Mill River(1)	56.1	82.5	100.0	126.0	
To Combined Sewers (1)	6.1	9.6	11.8	15.2	

(1) Total Discharge is based on sum of hydrographs

The Project will result in a decrease in stormwater runoff to the City combined sewers; see Section III, Utilities, for a further discussion of the combined sewers impacts and mitigation.

A summary of the total existing and proposed peak discharge to the Saw Mill River is shown in Table III. D-4.

Table III. D-4
Comparison of Total Peak Runoff to the Saw Mill River from Project Site

	Design Year Storm (CFS)					
	2-Year	10-Year	25-Year	100-Year		
Existing Condition	28.6	44.9	55.7	71.7		
Proposed Condition	56.1	82.5	100.0	126.0		
Difference	+27.5	+37.6	+44.3	54.3		

As shown in Table III. D-4, the increase of peak discharge to the Saw Mill River ranges from approximately 27.5 CFS for a 2-year design storm to 54.3 CFS for the 100-year storm. The 29-acre drainage area studied for the Project represents 0.1 percent of the total Saw Mill Drainage Basin. Based on information contained in the ACOE study entitled "Flood Control Study, Detailed Project Report, Saw Mill River, Nepera Park/Yonkers, Westchester County, New York July 1986", the peak discharge for the Saw Mill River occurs in Yonkers approximately 38 hours after the beginning of a storm event or approximately 20-24 hours after the peak rainfall. Since the Project is located proximate to the Saw Mill River, the runoff from the Project will enter the river very quickly. This peak discharge from the Project will occur at approximately 12 hours after the beginning of the rain event or within 10 to 20 minutes after the peak rainfall. When considering the project discharge combined with the upstream flow from the Saw Mill River, there will only be an increase of approximately 19 cfs during a 100-year storm. This nominal increase (1% +/-) will result in less than 0.1 changes to the water surface elevation and less than 0.1 feet per second change in the River flow downstream of the project (See Appendix B). Therefore, the minor increase in impervious area as a result of the Project and the diversion of runoff to the Saw Mill River will not have an impact on the drainage conditions downstream or adjacent to the Project.

• <u>Larkin Plaza</u> - The development of Larkin Plaza will result in approximately 0.68 acres of impervious area, including the daylighted portion of the Saw Mill River. This represents a 44 % reduction of impervious area over the existing conditions. A summary of the existing and proposed development peak runoff from the project site is shown in Table III.D- 5 (See Appendix 3C for Larkin Plaza calculations)

Table III. D-5 Comparison of Total Peak Runoff for Larkin Plaza Project Site

	Design Year Storm (CFS)				
	2-Year	10-Year	25-Year	100-Year	
Existing Condition	4.0	5.9	7.2	9.1	
Proposed Condition	2.8	4.8	6.1	8.0	
Difference	-1.2	-1.2	-1.1	-1.1	

4.2.2 Water Quality and Channel Protection

To obtain General Permit coverage typically requires conformance with the technical standards for storm water quantity and quality controls presented in the NYSDEC Design Manual. However, since this is a redevelopment project, water quality measures for the proposed Project will also be designed in accordance with NYSDEC Design manual, Chapter 9, "Redevelopment Projects".

Peak Storm Water Discharge

As shown above, the minor increase in impervious area as a result of the project and the diversion of runoff to the Saw Mill River will not increase the peak discharge in the Saw Mill River and the culvert/flume to the Hudson River. Thus, the peak discharge within the river downstream of the Project will not increase. Therefore, NYSDEC requirements for overbank flood control (10-year storm event) and extreme flood criteria (100-year storm event) do not apply to the sites that drain to the Saw Mill River.

Stream Channel Protection Volume

Stream Channel Protection Volume requirements as stated in Section 4.3 of the DEC Design Manual are designed to protect stream channels from erosion. However, the Stream Channel Protection Volume requirements are not applicable if discharge is to a tidal water or a fourth order stream. Section 4.7 of the DEC Design Manual states that streams can be classified according to their order in the network of streams in a watershed. A stream that is identified as a "blue-line" stream on USGS topo maps, and if it has no tributaries or branches it is defined as a first-order stream. When two first-order streams combine, a second-order stream is created, and so on. Based on a review of the USGS topo maps for the Saw Mill River watershed, over 6 "blue-line" streams enter the river north of Dobbs Ferry (See Appendix B). As the Saw Mill River in Yonkers is a greater than a fourth order stream and the Hudson River is a tidal water, the Project will not be required to comply

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with NYSDEC Stream Channel Protection Volume requirements for compliance with the General Permit.

Also, the analysis developed for the project has determined that project will not increase the velocity within the downstream portions of the Saw Mill River thereby not creating the potential for increase erosion.

Water Quality Requirements

Since this is a redevelopment project of previously developed area, water quality measures for the proposed Project shall be governed by the NYSDEC Chapter 9, Redevelopment Projects. The redevelopment standards require the following:

- The plan must capture and treat by implication of standard practices 25% of the water quality volume for the original impervious area, plus 100% of additional impervious area, from the disturbed site areas.
- The plan may propose use of non-standard alternate practices to treat 100% of the water quality volume from the disturbed area. The practice must be sized to capture and treat the water quality peak discharge rate associated with water quality volume.
- The use of standard practices should be targeted to treat areas with the greatest pollution potential (e.g. parking areas, roadways, etc.)

For the Project, underground treatment and/or filtration devices will be provided on all discharge points from the parking garages and site roads. In accordance with the DEC regulations, the water quality facilities are sized to capture and treat 90% of the average annual stormwater runoff volume. Diversion structures will be provided on runoff from buildings, pedestrian plazas, walkways, etc to divert the water quality flow to hydrodynamic systems prior to discharge. All systems will be constructed in accordance with NYSDEC requirements and treat Project generated stormwater rather than allow for the direct discharge of particulates/pollutants to the Saw Mill River. A summary of the water quality volume requirements for the project sites and the anticipated type of water quality devices are shown in Table III.D.6.

Table III. D-6
Preliminary Water Quality Volume and Description of WQ Devices

	Existing Impervious Area (Ac)	Proposed Impervious Area (Ac)	Water Quality Volume (Cubic Feet)	Water Quality Device Description
River Park Center	12.4	11.0	50,200	Hydrodynamic Systems To Treat WQ Flow Rate
Government Center	1.8	2.5	5, 300	Underground Filtration Device
Cacace Center	2.6	3.3	6,600	Underground Filtration Device

. A description of the water quality devices to be used for the project is described:

Underground Filtration Devices

The treatment of the water quality volume by standard practices will be accomplished through the design and implementation of underground filtration device systems. The required elements of these systems will be designed in accordance with Section 6.4 of the DEC Design Manual, and the <u>Design of Stormwater Filtering Systems</u>, December 1996, published by The Center for Watershed Protection. The devices will be sand filters, Storm Filter as provided by Contech or other similar products. The devices will be designed as "offline" practices, with flow diversion structures provided to divert the water quality volume to the filters for treatment. Larger flows will bypass the filters and be conveyed by the storm drainage pipe systems. The filters will provide 80% TSS removal and 40% TP removal. As stated in Table 7.4 of the DEC Design Manual, the filters will also provide good removal of nitrogen (>30% TN) and metals (>60% Metals) such as cadmium, copper, lead and zinc. Fair removal (35-70%) of bacteria such as coliform, streptococci and E. coli is expected.

Hydrodynamic Systems

Hydrodynamic systems such as gravity and vortex separators are devices that move water in a circular, centrifugal manner to accelerate the separation and deposition of primary sediment from the water. These measures shall include, but not be limited to, "proprietary" oil/grit separators/hydrodynamic chambers such as devices manufactured by Stormceptor® or Vortechs® systems. These devices contain both the isolation/diversion mechanism and a treatment chamber for capturing and treating the water quality flow. These devices can be installed in line with the storm drainage

system and contain a bypass for flows in excess of the water quality flow. These devices meet the goal of the DEC criteria to provide at least 80% removal of total suspended sediment (TSS) from the first flush post-construction runoff. As an additional treatment measure all catch basins will be provided with sumps.

All water quality measures will be designed as part of the Site Plan Approval plans and details. With the measures outline above in place and the implementation of the sediment control plan and onsite maintenance and impaction measures, the Project will comply with the requirements of the General Permit and the objectives of the City's Stormwater Management Program and the Flood, Erosion and Sediment Control regulations.

5.0 EROSION AND SEDIMENT CONTROL

5.1 Soil Erosion and Sediment Control Measures

During construction of the Project, the potential for soil erosion and sedimentation will be controlled through the use of temporary soil erosion and sediment control measures. These measures will be designed and installed in accordance with <u>New York Guidelines for Urban Erosion and Sediment Control</u> dated October 2005, and Chapter 56, Article XIII, "Flood, Erosion and Sediment Control of the City Code." The soil erosion and sediment control plan will minimize the downstream erosion by controlling runoff at its source, minimizing runoff from disturbed areas and de-concentrating storm water runoff. Temporary and permanent stabilization methods will be implemented before construction begins and will be continuously modified throughout the project to provide the best methods for stormwater management and pollution prevention.

Phasing of activities shall be as follows:

Pre-Construction Activities

- Identify all natural resources and mark and protect them as necessary i.e. trees, vegetation.
- Identify on-site and downstream surface water bodies and install controls to protect them from sedimentation.
- Establish temporary stone construction entrance pads to capture mud and debris from the tires of construction vehicles.
- Install perimeter sediment controls such as silt fence as shown on the project plans.
- All earth disturbances during this phase should be limited to work necessary to install erosion and sedimentation controls.

During Construction Activities

- Install runoff and drainage controls as shown on the project plans and as necessary.
 These controls should reduce run-off flow rates and velocities as well as divert off site and clean run-off.
- Stabilize the conveyance system (i.e. ditches, swales, berms etc.) by seeding, mulching, installing rock check dams.
- Stabilize all stormwater runoff outlets as shown on the project plans and as necessary.
- Stabilization measures should be initiated as soon as practical in portions of the site where construction activities have temporarily or permanently ceased, but in no case more than 14 days. Where activities will resume within 21 days in that portion of the site, measures need not be initiated.
- Limit soil disturbance and exposure of bare earth to a minimum.
- All topsoil stockpiles should be staged in an area away from surface waters and storm drains and should be protected and stabilized.
- Construction vehicles shall enter and exit the site at the stabilized construction entrance. The construction entrances will be maintained during the life of the construction and repaired and/or cleaned periodically to ensure proper function.
- Water trucks will be used as needed during construction to reduce dust generated on the site. The contractor will provide dust control in compliance with applicable local and state dust control regulations.
- At any location where surface run-off from disturbed or graded areas may flow offsite, sedimentation control measures must be installed to prevent sedimentation from being transported.
- Regular inspections and maintenance should be performed as described in the following section.

Post-Construction Activities

- Identify the permanent structural or non-structural practices that will remain on the site.
- Provide an Operation & Maintenance (O&M) manual to the Owner who is expected to conduct the necessary O&M over the life of the structures.

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5.2 Other Pollutant Controls

Paints and Solvents

During construction, temporary structures such as construction trailers may be moved on site to store items such as paints, solvents and gasoline pertinent to the continuation of construction activities. The intention of these structures is to shelter such items and reduce the potential of entering the stormwater runoff due to construction activities. After use, solvents shall be disposed of in approved containers and removed from site at scheduled intervals.

Fuels

Fuel for construction equipment shall either be obtained from a licensed distributor of petroleum products or from an approved above ground storage tank on site. Fuel from construction vehicles may come into contact with stormwater when vehicles are stored outside. Good housekeeping and preventative maintenance procedures shall be implemented to ensure fuel spills and leaks are minimized during refueling and storage.

Temporary Facilities

Temporary sanitary facilities may be located on site for construction workers. This facility shall be located in an accessible and visible location. A waste management company may be contracted to arrive on site and provide the routine pumping and sanitization of the facility.

Solid Waste

No solid materials are allowed to be discharged from the site with stormwater. All solid waste shall be collected and placed in containers. The containers will be emptied periodically by a contract trash disposal service and hauled away from the site.

5.3 Best Management Practices

Throughout construction, care shall be taken to ensure sediment does not enter surface water bodies and chemicals do not enter stormwater, potentially contaminating surface and groundwater supplies. The following Best Management Practices (BMP) shall be observed to maintain responsible environmental practices on the construction site.

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Good Housekeeping

Good housekeeping is essential to reducing the risk of contaminating runoff waters during every stage of construction. The General Contractor shall ensure supervisors train each employee in good housekeeping practices as they pertain to the implementation of this SWPPP.

Immediately following mobilization, the General Contractor shall take an inventory of all equipment and containers containing hazardous or toxic materials and submit this inventory to the Owner to keep on-site with this Stormwater Pollution Prevention Plan. This inventory shall be updated regularly to reflect changes in the quantity or type of hazardous and toxic materials stored on site. In the event of a spill, the Spill Response Team can refer to the inventory if the contents of the spill are unknown.

All equipment shall be operational while it is stored on site. Inspections shall be conducted regularly to ensure all equipment is free of leaks and that oil and grease are not in contact with soils or stormwater. Portable equipment such as chain saws, drills as well as hand tools must be placed within a trailer or under cover at the end of each workday.

A storage area shall be designated on-site where all hazardous or toxic materials are stored. Each employee shall return the materials to the designated storage area following use. Chemicals, including oil, grease, solvents and detergents shall be stored on-site in approved containers only. Used chemicals shall be disposed of in refuse containers and removed periodically. Containers shall be regularly inspected to ensure the integrity of the container and seals to prevent leaks.

5.4 Construction Sequence Scheduling

A phased construction sequence schedule of the Project will limit the acreage of exposed soils at any given time. Since the project site disturbance will be greater than 5-acres, the construction sequence will require the approval of the NYSDEC prior to the filing of the NOI. Limiting the exposed soils will reduce the amount of sediments in runoff water and ultimately preserve the quality of surface waters. The construction phasing method selected will be designed to combine development with responsible land management as well as protection of sensitive environments both within the proposed Project and the surrounding area.

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5.5 Implementing the SWPPP

The General Permit requires that site assessment and inspections for all construction activities in excess of one (1) acre. The site assessment and inspections insure the implementation of the SWPPP to retain surface water quality and prevent sediment-laden runoff from entering rivers, streams, estuaries, wetlands and other sensitive environments.

The site assessment and inspections required for this project will include the following:

- 1. The operator/owner shall have a "qualified professional" conduct an assessment of the site prior to the commencement of construction and certify in an inspection report that the appropriate erosion and sediment controls described in the SWPPP and required by the General Permit have been adequately installed or implemented to ensure overall preparedness of the site for the commencement of construction.
- 2. Following the commencement of construction, site inspections shall be conducted by the qualified professional at least every seven (7) calendar days and within 24 hours of the end of a storm event of 0.5 inches or greater. During each inspection, the qualified professional shall provide a written report identifying all disturbed site areas and drainage pathways, areas of the site that have undergone temporary or permanent stabilization, indicate all disturbed site areas that have not undergone active site work during the previous 14-day period, all maintenance requirements for sediment control measures and all deficiencies that are identified with the implementation of the SWPPP.
- 3. Prior to filing of the Notice of Termination or the end of permit term, the qualified professional perform a final site inspection. The qualified professional shall certify that the site has undergone final stabilization using either vegetative or structural stabilization methods and that all temporary erosion and sediment controls (such as silt fencing) not needed for long-term erosion control have been removed.

6.0 CONCLUSION

The proposed stormwater management system reduces and/or eliminates the impacts of the proposed development by controlling and treating stormwater through the use of drainage ditches and channels, storm sewer piping, and a stormwater management basin. The stormwater management system will function adequately and will not adversely effect

adjacent or downstream properties provided it is constructed and maintained as outlined in this plan and as shown on the site plans.

This report is respectfully submitted in accordance with our contract, and is to the best of our knowledge accurate and complete. Any questions regarding its content may be directed to the undersigned.

Respectfully submitted by,

The Office of

McLaren Engineering Group

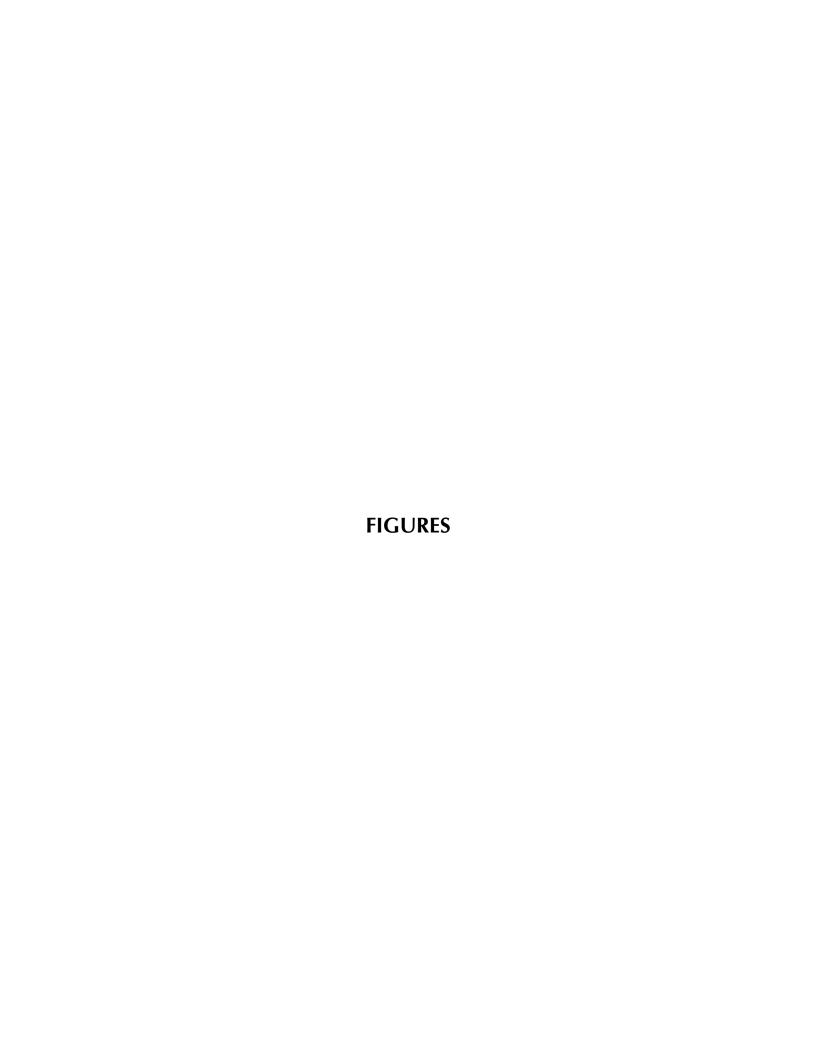
M.G. McLAREN, P.C.

Steven L. Grogg, P.E.

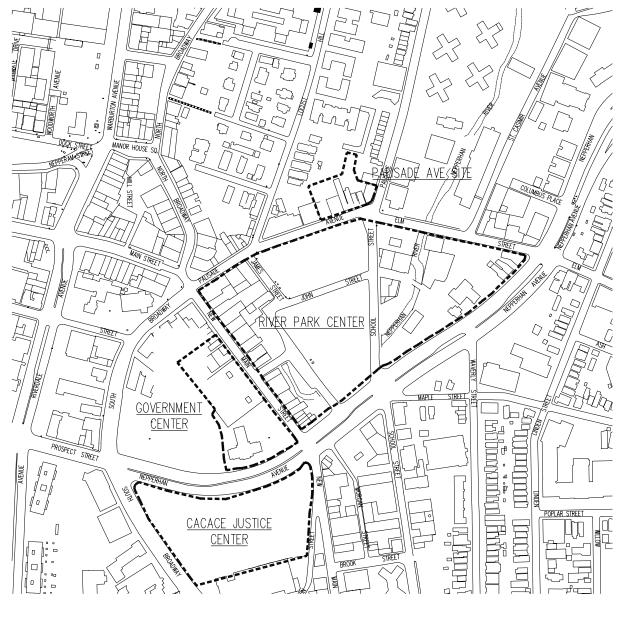
Chief Site - Civil Division

SLG/rjk

P:\Proj106\106100\8. Technical (Presentations, Rpts, Calcs, Specs)\Reports\Current\Preliminary SWPPP Report Revised12-12-07.doc







----- DRAINAGE BOUNDARY

REV 02/12/07



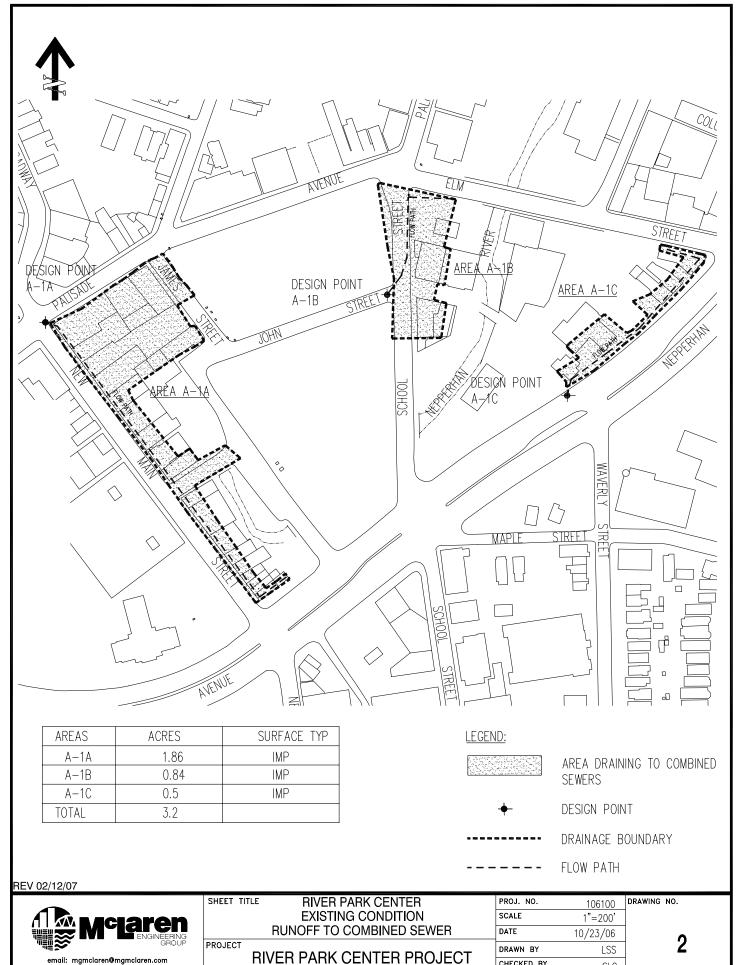
	email:	mg	mclare	n@mg	mclare	1.00	m	
100	Snake	Hill	Road,	West	Nyack,	NY	10994	
Tel.	(845)	353	-6400	Fax.	(845)	353	-6509	

SHEET TITL	EXISTING CONDITION
	OVERALL PROJECT AREA MAP
PROJECT	

RIVER PARK CENTER PROJECT
YONKERS, NEW YORK

PROJ. NO.	106100
SCALE	1"=400'
DATE	10/23/06
DRAWN BY	LSS
CHECKED BY	SLG

DRAWING	S N	0.	
		4	
		J	
1 (ΣF	<u>9</u>	SHTS

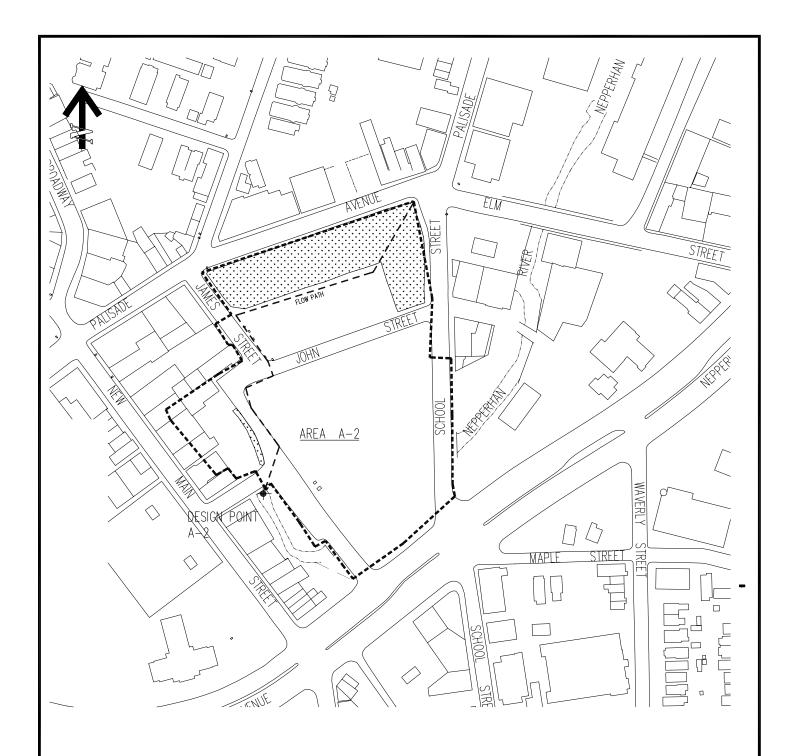


email: mgmclaren@mgmclaren.com 100 Snake Hill Road, West Nyack, NY 10994 Tel. (845) 353-6400 Fax. (845) 353-6509

YONKERS, NEW YORK

PROJ. NO.	106100	DF
SCALE	1"=200'	
DATE	10/23/06	
DRAWN BY	LSS	
CHECKED BY	SLG	

<u>2</u> OF <u>9</u> SHTS



AREA A-2				
SURFACE TYPE	AREA (ACRES)			
IMPERVIUOS	5.45			
PERVIOUS	1.23			
TOTAL	6.68			

PERVIOUS AREA

|

DESIGN POINT

DRAINAGE BOUNDARY

---- FLOW PATH

REV 02/12/07



email: mgmclaren@mgmclaren.com 100 Snake Hill Road, West Nyack, NY 10994 Tel. (845) 353-6400 Fax. (845) 353-6509 SHEET TITLE RIVER PARK CENTER
EXISTING CONDITION
RUNOFF STORM DRAIN/RIVER

PROJECT

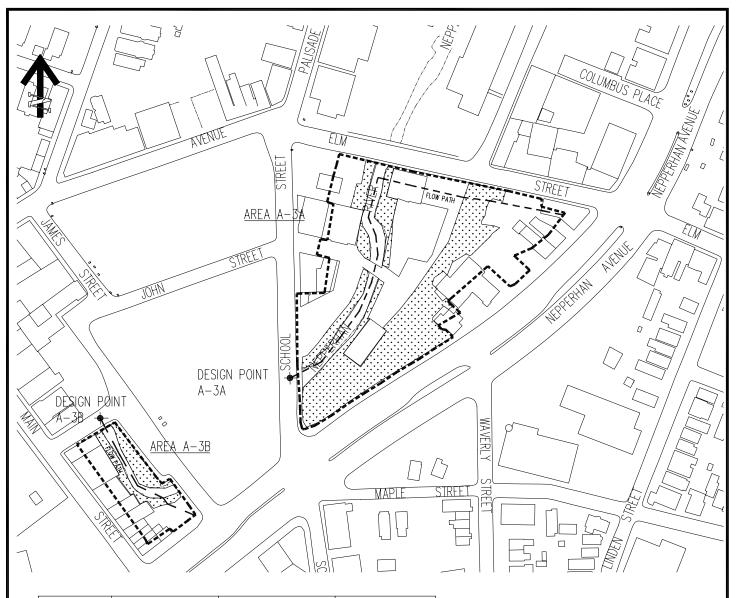
RIVER PARK CENTER PROJECT YONKERS, NEW YORK

PROJ. NO.	106100	С
SCALE	1"=200'	
DATE	10/23/06	
DRAWN BY	LSS	
CHECKED BY	SIG	

DRAWING NO.

3

3 OF 9 SHTS



AREAS	IMPERVIUOS (Ac) PERVIOUS (Ac)		TOTAL (Ac)
A-3A	2.32	1.45	3.77
A-3B	0.40 0.19		0.59
TOTAL			4.36

PERVIOUS AREA

+

DESIGN POINT

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DRAINAGE BOUNDARY

---- FLOW PATH

REV 02/12/07



email: mgmclaren@mgmclaren.com 100 Snake Hill Road, West Nyack, NY 10994 Tel. (845) 353-6400 Fax. (845) 353-6509 SHEET TITLE RIVER PARK CENTER
EXISTING CONDITION
RUNOFF DIRECTLY TO RIVER

PROJECT RIVER PA

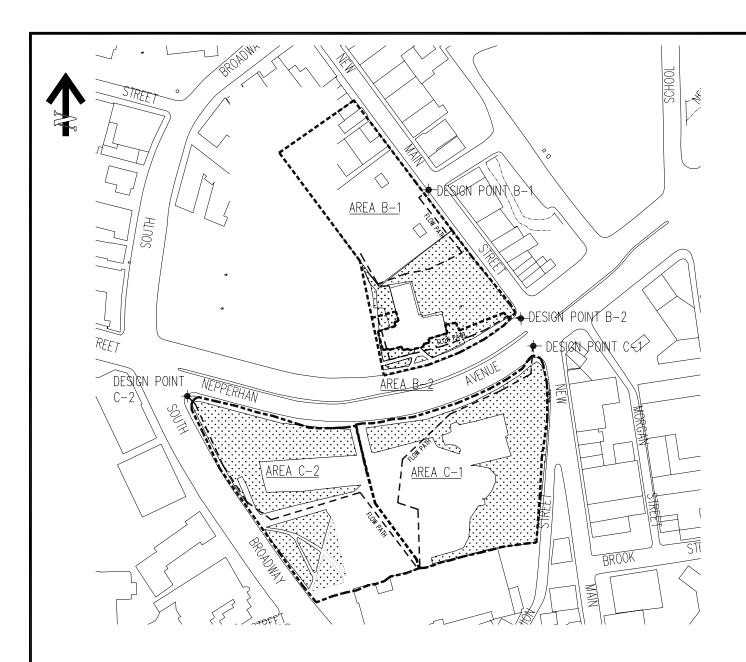
RIVER PARK CENTER PROJECT YONKERS, NEW YORK

PROJ. NO.	106100	E
SCALE	1"=200'	
DATE	10/23/06	
DRAWN BY	LSS	
CHECKED BY	SLG	

DRAWING NO.

4

<u>4</u> OF <u>9</u> SHTS



AREAS	IMPERVIUOS (Ac) PERVIOUS (Ac)		TOTAL (Ac)
B-1	1.47	1.03	2.50
B-2	0.15	0.17	0.32
C-1	1.36	1.22	2.58
C-2	-2 1.19 1.16		2.35
	7.75		

PERVIOUS AREA

-

DESIGN POINT

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DRAINAGE BOUNDARY

FLOW PATH

REV 02/12/07



email: mgmclaren@mgmclaren.com 100 Snake Hill Road, West Nyack, NY 10994 Tel. (845) 353-6400 Fax. (845) 353-6509 EXISTING CONDITION
CACACE AND GOVERNMENT CENTERS

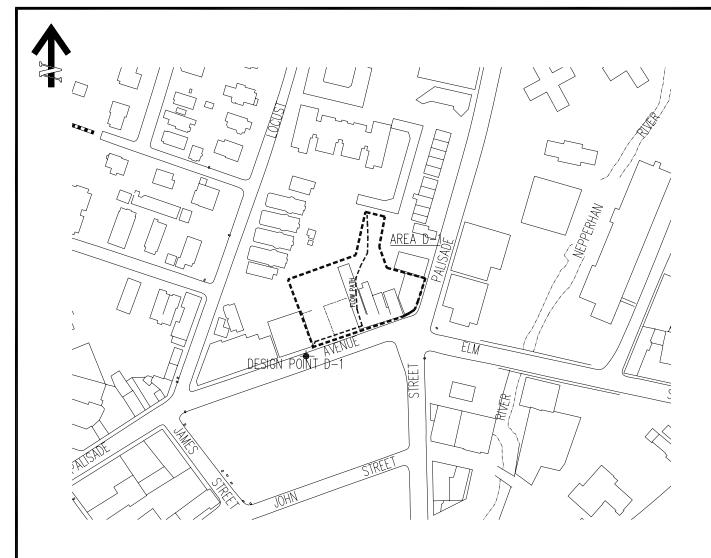
PROJECT

RIVER PARK CENTER PROJECT YONKERS, NEW YORK

PROJ. NO.	106100	I
SCALE	1"=200'	
DATE	10/23/06	
DRAWN BY	LSS	
CHECKED BY	SLG	

DRAWING NO.

<u>5</u> OF <u>9</u> SHTS



AREAS	IMPERVIUOS (Ac)	GRAVEL (Ac)	TOTAL (Ac)
D-1	0.52	0.35	0.87

♦ DESIGN POINT

DRAINAGE BOUNDARY

---- FLOW PATH

REV 02/12/07



email: mgmclaren@mgmclaren.com							
100	Snake	Hill	Road,	West	Nyack,	NY	10994
Tel.	(845)	353	-6400	Fax.	(845)	353	-6509

SHEET TITLE	EXISTING CONDITION	
	PALISADE AVENUE SITE	
PRO IECT		Ī

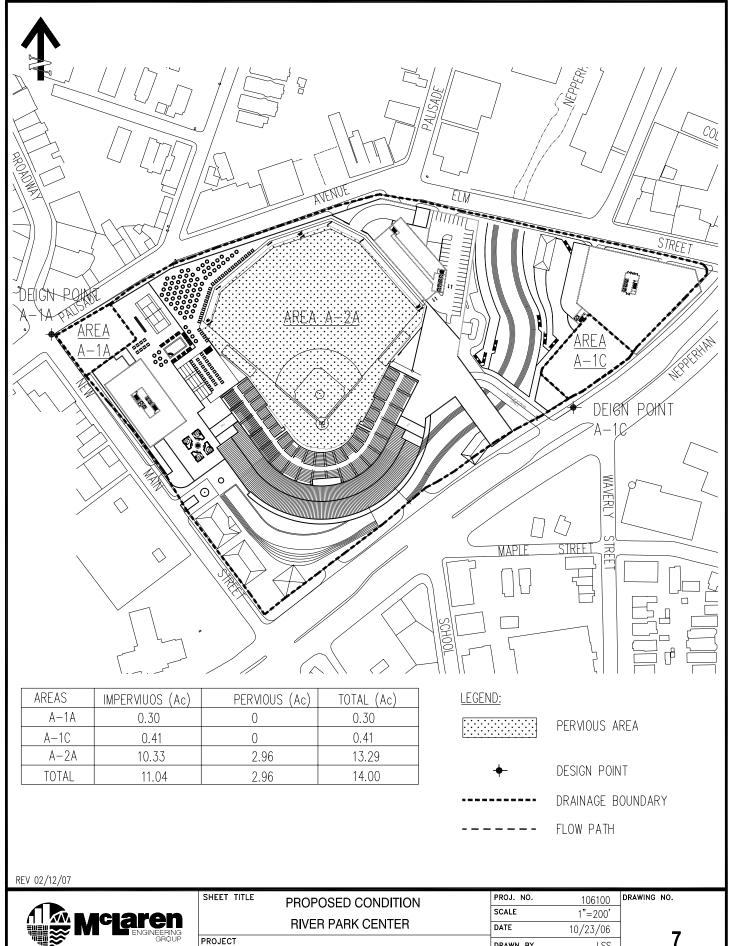
RIVER PARK CENTER PROJECT
YONKERS, NEW YORK

PROJ. NO.	106100
SCALE	1"=200'
DATE	10/23/06
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DRAWING	NO.
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6

<u>6</u> OF <u>9</u> SHTS

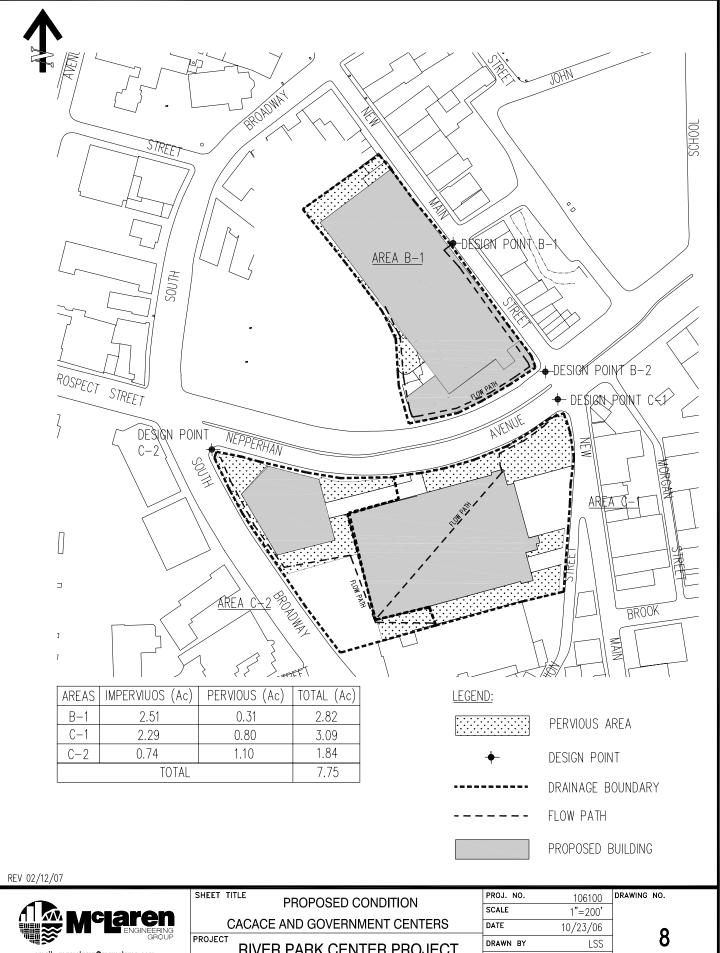


email: mgmclaren@mgmclaren.com 100 Snake Hill Road, West Nyack, NY 10994 Tel. (845) 353-6400 Fax. (845) 353-6509

RIVER PARK CENTER PROJECT YONKERS, NEW YORK

DRAWN BY LSS CHECKED BY SLG

<u>_7_</u> OF <u>_9</u>_ SHTS



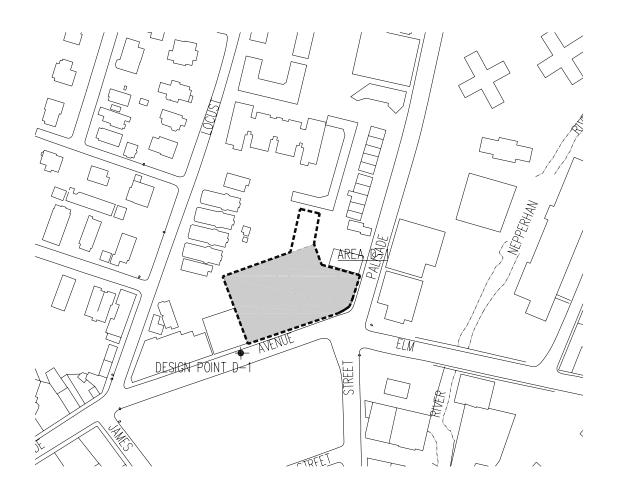
email: mgmclaren@mgmclaren.com 100 Snake Hill Road, West Nyack, NY 10994 Tel. (845) 353-6400 Fax. (845) 353-6509

RIVER PARK CENTER PROJECT YONKERS, NEW YORK

CHECKED BY SLG

<u>8</u> OF <u>9</u> SHTS





AREAS	IMPERVIUOS (Ac)	PERVIOUSL (Ac)	TOTAL (Ac)
D-1	0.78	0.09	0.87

LEGEND:

DESIGN POINT

----- DRAINAGE BOUNDARY

PROPOSED BUILDING

REV 02/12/07



email:	mg	mclare	n@mg	mclare	n.coi	m
				Nyack, (845)		

SHEET TITLE	PROPOSED CONDITION				
	PALISADE AVENUE SITE				

PROJECT

RIVER PARK CENTER PROJECT YONKERS, NEW YORK

PROJ. NO.	106100
SCALE	1"=200'
DATE	10/23/06
DRAWN BY	LSS
CHECKED BY	SLG

DRAWING NO.

9

<u>9</u> OF <u>9</u> SHTS

APPENDIX A EXISTING CONDITIONS

New York State Department of Environmental Conservation

Division of Environmental Permits, Region 3 21 South Putt Corners Road, New Paltz, New York 12561-1620 Phone: (845) 256-3054 • FAX: (845) 255-3042

Website: www.dec.ny.gov



Alexander B. Grannis Commissioner

October 29, 2007

PK Environmental Attn: Patrick McNamara 205 Main Street Chatham, NJ 07928

RE:

Stream Classification/Resource Jurisdiction

Saw Mill River Town of Yonkers Westchester County

Dear Mr. McNamara:

	Based upon our review of your inquiry dated October 16, 2007, we offer the	e following comments	•
	PROTECTION OF WATERS		
	The following stream(s)/pond(s)/waterbody(ies) is(are) located within o	or near the site you in	dicated:
	Name Class DEC Water Index Number	Status	
	Saw Mill River [C] H-4	[Protected, non-protected, non-prote	navigable]
1	A Protection of Waters permit is required to physically disturb the bed of any streams identified above as "protected." A permit is not required to protected" streams.	or banks (up to 50 fee o disturb the bed or t	t from stream) panks of "non-
	☐ A Protection of Waters permit is required for any excavation or filling be waterbodies identified above as "navigable."	elow the mean high w	ater line of any
	There are no waterbodies that appear on our regulatory maps at the Therefore, if there is a stream or pond outlet present at the site with year-rour of the watercourse into which it feeds, Protection of Waters permit is/is not required. If there is a stream or pond intermittently (seasonally), it is not protected, and a Protection of Waters p	nd flow, it assumes th , Class " d outlet present at the	e classification
K	If a permit is not required, please note, however, you are still responsible for any stream or waterbody. Care shall be taken to stabilize any disturbed are all necessary precautions shall be taken to prevent contamination of the str fuels, solvents, lubricants, or any other pollutant associated with the project	eas promptly after co ream or waterbody by	nstruction, and
	FRESHWATER WETLANDS		
	Your project/site is near or in Freshwater Wetland, C Freshwater Wetlands permit is required for any physical disturbance within foot adjacent area. To have the boundary delineated, please read the atta	n these boundaries or	e aware that a within the 100

	Your project/site is not within a New York State protected Freshwater Wetland. However, please con your town officials and the United States Army Corps of Engineers in New York City, telephone (917) 790-8 (Westchester/Rockland Counties), or (917) 790-8411 (other counties), for any permitting they might requ	3511
	STATE-LISTED SPECIES	
	DEC has reviewed the State's Master Habitat Databank (MHDB) records. We have determined that the is located within or near record(s) of the following state-listed species: If your inquirelated to a specific development project, additional evaluation of the potential impacts of this project relates the sensitive resource(s) identified by this review, may be required. Please contact the person noted below	iry is ed to
	No records of sensitive resources were identified by this review.	
×	OTHER: Please note that Stream classification is limited to the area identified on the supplied ma	
	Stream classification changes to protected status further upstream & downstream of this specific lo	<u>ucati</u> on.
	Please note that this letter only addresses the requirements for the following permits from the Department Freshwater Wetlands Master Habitat Databank Other: Protection of Waters and that other permits from this Department or other agencies may be required for projects conducted on property now or in the future. Also, regulations applicable to the location subject to this determine occasionally are revised and you should, therefore, verify the need for permits if your project is delayed postponed. This determination regarding the need for permits will remain effective for a maximum of one unless you are otherwise notified. Applications may be downloaded from our website at www.dec.ny.gov u "Programs" then "Division of Environmental Permits."	n this ation ed or year
	Please contact this office if you have questions regarding the above information. Thank you.	
	John W. Petronella Division of Environmental Permits Region 3, Telephone No. 845/256-3031	
	Information/Permit Materials/Regulations/Map (\(\frac{1}{2} \) Quadrangle) Attached. Web page information NYC DEP Contact Information (this site is within the NYC Watershed). cc:	
Yen.	NOTE: Regarding erosion/sedimentation control requirements: Stormwater discharges now require a SPDES Stormwater permit from this Department if they either:	1*

occur at industrial facilities and contain either toxic contaminants or priority pollutants OR
 result from construction projects involving the disturbance of one (1) or more acres of land.

individual permit. If you believe your project would be covered under one of the General Permits and does not require any other DEC permits you may apply for coverage by filing a Notice of Intent with NYSDEC, Division of Water, 625 Broadway, Albany NY 12233-3505, (forms & permits available from this office or DEC Website at www.dec.ny.gov or call 518-402-8109). If your project involves other DEC permits, please contact the regional Division of Environmental Permits office (see above).

Your project may be covered under one of two Statewide General Permits or may require an

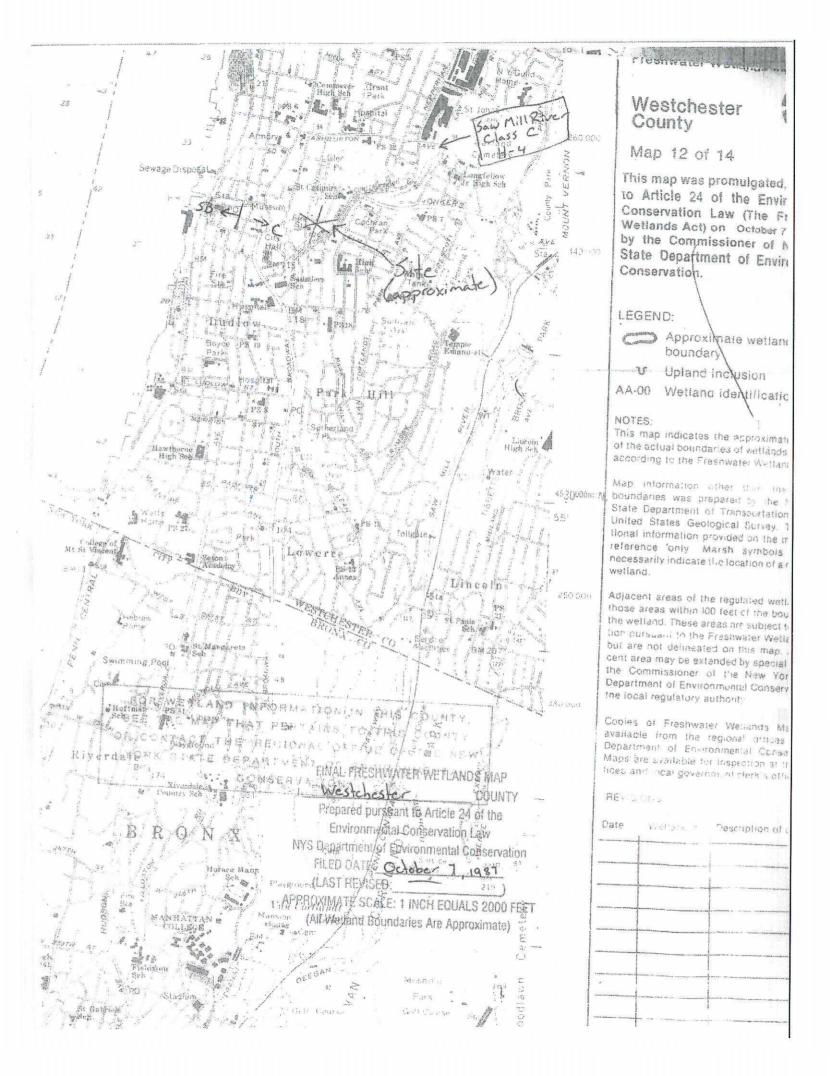
Date: October 29, 2007

S:\PERMITS\FORMS D-1\GENJURSD\General Response Letter(an) wpd

RE:

Saw Mill River

Rev. 7/07



MEG 106100 EXIST COND RUN-1: CACACE, GOVT, AND RIVER PARK CENTERS Westchester County, New York

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Area Concentration		Receiving Reach	Sub-Area Description		
A-1A	1.86	0.109	95	Outlet	R-PARK TO COMB SEW		
A-1B	.84	0.100	95	Outlet	R-PRK TO COMB SEW		
A-1C	.50	0.100	95	Outlet	R-PRK TO COMB SEW		
A-2	6.68	0.320	91	Outlet	R-PRK TO STORM DR		
A-3A	3.77	0.100	88	Outlet	R-PRK TO RIVER		
A-3B	.59	0.119	89	Outlet	R-PRK TO RIVER		
B-1	2.50	0.100	91	Outlet	G-CEN GAR TO COMB SEWER		
B-2	.32	0.100	88	Outlet	G-CEN GAR TO STORM DR		
C-1	2.58	0.100	89	Outlet	CACACE JUS CEN TO STORM D		
C-2	2.35	0.100	89	Outlet	CACACE JUS CEN SEWER		

Total Area: 21.99 (ac)

B. SANTOS MEG 106100
EXIST COND RUN-1: CACACE, GOVT, AND RIVER PARK CENTERS
Westchester County, New York

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Perimeter	Velocity (ft/sec)	Travel Time (hr)
A-1A SHEET SHALLOW CHANNEL	50 100 620	0.0100 0.0290	0.011 0.025			2.000	0.015 0.008 0.086
				Ti	me of Concer		0.109
A-1B SHEET SHALLOW CHANNEL		0.0200 0.0250	0.011 0.025			2.000	0.019 0.011 0.012
				Ti	me of Concer		0.100
A-1C SHEET SHALLOW CHANNEL		0.0200 0.0330	0.011 0.025			3.000	0.019 0.008 0.030
				Tir	me of Concen		0.100
A-2 SHEET SHALLOW SHALLOW CHANNEL	80	0.0556 0.0500 0.0300	0.410 0.050 0.025			2.000	0.230 0.006 0.024 0.060
				Tir	me of Concen		0.320
A-3A SHEET SHALLOW CHANNEL		0.0500 0.0500	0.150 0.050	Tin	me of Concen	2.000	0.072 0.001 0.026
				1 11	me or concen		
A-3B SHEET SHALLOW SHALLOW CHANNEL CHANNEL	46 108 55 171 369	0.0100 0.0830 0.4300	0.011 0.025 0.050			1.000	0.014 0.005 0.001 0.048 0.051
				Tin	ne of Concen	tration ==	0.119
B-1 SHEET SHALLOW CHANNEL	99 150 300	0.0200 0.0700	0.011 0.050			7.000	0.019 0.010 0.012
WinTR-55, Ve	rsion 1.00	.08	Page	Tin	ne of Concen		0.1 9:40:28 AM
B-2 SHEET SHALLOW SHALLOW CHANNEL	99 150 150 200	0.0100 0.0200 0.2000	0.011 0.025 0.050	•		10.000	9:40:28 AM 0.025 0.014 0.006 0.006

MEG 106100
EXIST COND RUN-1: CACACE, GOVT, AND RIVER PARK CENTERS
Westchester County, New York B. SANTOS

Sub-Area Time of Concentration Details (continued)

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
				Ti	me of Cond	centration	0.1
C-1							
SHEET	99	0.0100	0.011				0.025
SHALLOW	150	0.0200	0.025				0.014
SHALLOW	200	0.7000	0.025				0.003
CHANNEL	260					7.000	0.010
				Ti	me of Cond	entration	0.1
C-2							
SHEET	99	0.0200	0.011				0.019
SHALLOW	150	0.0500	0.025				0.009
CHANNEL	250					5.000	0.014
				Ti	me of Conc	entration	0.1

B. SANTOS MEG 106100
EXIST COND RUN-1: CACACE, GOVT, AND RIVER PARK CENTERS
Westchester County, New York

Sub-Area Land Use and Curve Number Details

Sub-Area Gentifier	Land Use	Hydrologic Soil Group	Area (ac)	Number
	& business	D	1.86	95
Total Area	/ Weighted Curve Number		1.86	95 ==
1-1B Commercial	& business	D	.84	95
Total Area	/ Weighted Curve Number		.84	95 ==
-1C Commercial	& business	D	.5	95
Total Area	/ Weighted Curve Number		. 5	95 ==
-2 Open space; Paved; open	grass cover > 75% (good ditches (w/right-of-way)	D D	1.23 5.45	80 93
Total Area	/ Weighted Curve Number		6.68	91 ==
	grass cover > 75% (good ditches (w/right-of-way)	d) D D	1.45 2.32	80 93
Total Area	/ Weighted Curve Number		3.77	88
	grass cover > 75% (good ditches (w/right-of-way)	d) D	.19	80 93
Total Area	/ Weighted Curve Number		.59	89 ==
	grass cover > 75% (gooding lots, roofs, driveways	d) D	1.03 1.47	80 98
Total Area	/ Weighted Curve Number		2.5	91 ==
	grass cover > 75% (goo ng lots, roofs, driveways	d) D D	.17 .15	80 98
Total Area	/ Weighted Curve Number		.32	88
	grass cover > 75% (goo	d) D D	1.22 1.36	80 98
Total Area	/ Weighted Curve Number		2.58	89
	grass cover > 75% (goo ng lots, roofs, driveways	d) D	1.16 1.19	80 98
Total Area	/ Weighted Curve Number		2.35	89
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MEG 106100 EXIS COND RUN-2 : Palisade Ave Site Westchester County, New York

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
D-1	-87	0.100	95	Outlet	R-PRK GAR TO COMB SEW

Total Area: .87 (ac)

MEG 106100 EXIS COND RUN-2 : Palisade Ave Site Westchester County, New York

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
D-1							
SHEET	40	0.0100	0.011				0.012
SHALLOW	200	0.0700	0.025				0.010
				Ti	me of Conce	ntration	0.100

MEG 106100 EXIS COND RUN-2 : Palisade Ave Site Westchester County, New York

Sub-Area Land Use and Curve Number Details

Sub-A Identi		Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
D-1	Commercial & business	D	.87	95
	Total Area / Weighted Curve Number		.87	95
				===

lyd.	Hydrograph type (origin)		Inflow				Peak Out	flow (cfs)				Hydrograph
No.		Hyd(s)	1-Yr	2-Yr	3-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	description	
1	SCS Runoff		12.000.000.00	5.223			7.668	9.284	1-2-1-1-1	11.70	Area A-1A	
2	SCS Runoff			2.359			3.463	4.193		5.282	Area A-1B	
3	SCS Runoff			1.404			2.061	2.496		3.144	Area A-1C	
4	SCS Runoff			14.01			21.51	26.48		33.87	Area A-2A	
5	SCS Runoff			8.676			13.77	17.15		22.19	Area A-3A	
6	SCS Runoff			1.404			2.201	2.729		3.516	Area A-3B	
7	SCS Runoff			6.336			9.697	11.92		15.22	Area B-1	
8	SCS Runoff			0.736			1.168	1.456		1.884	Area B-2	
9	SCS Runoff			6.141			9.624	11.93		15.37	Area C-1	
10	SCS Runoff			5.594			8.766	10.87		14.00	Area C-2	
11	SCS Runoff			2.443			3.587	4.343		5.471	Area D-1	
12	Combine	4, 5, 6,		22.03			34.25	42.35		54.45	River Park Combined to Saw Mill r	
13	Combine	1, 2, 3,		8.986			13.19	15.97		20.12	River Park Combined to Combined	
14	Combine	1, 2, 3, 4,	5,- 6,	30.73			47.27	58.24		74.58	Total River Park Center	
15	Combine	7, 8,		7.072			10.86	13.37		17.11	Total Government Center	
16	Combine	9, 10,		11.73			18.39	22.80		29.38	Total Cacace Center	
17	Combine	4, 5, 6, 8,	9,	28.62			44.87	55.65		71.71	Existing Combined to SMR	
18	Combine	1, 2, 3, 7,	10 , 11,	23.36			35.24	43.10		54.82	Total Existing to Combined Sewer	

Proj. file: Saw Mill River Flow Existing Conditions.gpw

Tuesday, Dec 4, 2007

lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph description
1	SCS Runoff	5.223	3	726	0.427				Area A-1A
2	SCS Runoff	2.359	3	726	0.193	<u> </u>			Area A-1B
3	SCS Runoff	1.404	3	726	0.115				Area A-1C
4	SCS Runoff	14.01	3	732	1.459				Area A-2A
5	SCS Runoff	8.676	3	726	0.668				Area A-3A
i	SCS Runoff	1.404	3	726	0.109				Area A-3B
S.	SCS Runoff	6.336	3	726	0.496	-		A CONTRACTOR OF THE PARTY OF TH	Area B-1
	SCS Runoff	0.736	3	726	0.057		: 		Area B-2
Ü	SCS Runoff	6.141	3	726	0.475			# 	Area C-1
0	SCS Runoff	5.594	3	726	0.433				Area C-2
1	SCS Runoff	2.443	3	726	0.200				Area D-1
2	Combine	22.03	3	729	2.236	4, 5, 6,			River Park Combined to Saw Mill r
3	Combine	8.986	3	726	0.735	1, 2, 3,			River Park Combined to Combined
4	Combine	30.73	3	726	2.970	1, 2, 3, 4,	5, 6,		Total River Park Center
5	Combine	7.072	3	726	0.553	7, 8,			Total Government Center
6	Combine	11.73	3	726	0.908	9, 10,			Total Cacace Center
7	Combine	28.62	3	726	2.767	4, 5, 6, 8,	9,		Existing Combined to SMR
8	Combine	23.36	3	726	1.864	1, 2, 3, 7,	10, 11 ,	() 	Total Existing to Combined Sewer
Sav	v Mill River F	low Exis	ting Cor	ditions.g	wReturn F	Period: 2 Y	ear	Tuesday, I	Dec 4, 2007

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph description
1	SCS Runoff	7.668	3	726	0.642				Area A-1A
2	SCS Runoff	3.463	3	726	0.290			<u> </u>	Area A-1B
3	SCS Runoff	2.061	3	726	0.173			THE BOOKERS.	Area A-1C
4	SCS Runoff	21.51	3	732	2.286				Area A-2A
5	SCS Runoff	13.77	3	726	1.081				Area A-3A
3	SCS Runoff	2.201	3	726	0.174				Area A-3B
7	SCS Runoff	9.697	3	726	0.778				Area B-1
3	SCS Runoff	1.168	3	726	0.092				Area B-2
9	SCS Runoff	9.624	3	726	0.760				Area C-1
10	SCS Runoff	8.766	3	726	0.693				Area C-2
11	SCS Runoff	3.587	3	726	0.300				Area D-1
12	Combine	34.25	3	729	3.540	4, 5, 6,			River Park Combined to Saw Mill r
13	Combine	13.19	3	726	1.105	1, 2, 3,			River Park Combined to Combined
14	Combine	47.27	3	726	4.645	1, 2, 3, 4,	5, 6,		Total River Park Center
15	Combine	10.86	3	726	0.869	7, 8,			Total Government Center
16	Combine	18.39	3	726	1.453	9, 10,			Total Cacace Center
17	Combine	44.87	3	726	4.392	4, 5, 6, 8,	9,		Existing Combined to SMR
18	Combine	35.24	3	726	2.876	1, 2, 3, 7,	10, 11 ,		Total Existing to Combined Sewer
0	/ Mill River F			450	D-4			Tuesday, I	

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph description
1	SCS Runoff	9.284	3	726	0.786				Area A-1A
2	SCS Runoff	4.193	3	726	0.355	2252			Area A-1B
3	SCS Runoff	2.496	3	726	0.211				Area A-1C
4	SCS Runoff	26.48	3	732	2.846				Area A-2A
5	SCS Runoff	17.15	3	726	1.362				Area A-3A
6	SCS Runoff	2.729	3	726	0.218				Area A-3B
7	SCS Runoff	11.92	3	726	0.968				Area B-1
3	SCS Runoff	1.456	3	726	0.116				Area B-2
9	SCS Runoff	11.93	3	726	0.954				Area C-1
0	SCS Runoff	10.87	3	726	0.869				Area C-2
1	SCS Runoff	4.343	3	726	0.368				Area D-1
2	Combine	42.35	3	729	4.426	4, 5, 6,			River Park Combined to Saw Mill r
3	Combine	15.97	3	726	1.353	1, 2, 3,			River Park Combined to Combined
4	Combine	58.24	3	726	5.779	1, 2, 3, 4,	5, 6,		Total River Park Center
5	Combine	13.37	3	726	1.084	7, 8,			Total Government Center
6	Combine	22.80	3	726	1.824	9, 10,			Total Cacace Center
7	Combine	55.65	3	726	5.497	4, 5, 6, 8,	9,		Existing Combined to SMR
8	Combine	43.10	3	726	3.558	1, 2, 3, 7,	10, 11 ,		Total Existing to Combined Sewer
Sav	v Mill River F	low Exis	ting Con	ditions.g	wReturn F	Period: 25	Year	Tuesday, I	Dec 4, 2007

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph description
1	SCS Runoff	11.70	3	726	1.003				Area A-1A
2	SCS Runoff	5.282	3	726	0.453				Area A-1B
3	SCS Runoff	3.144	3	726	0.270				Area A-1C
4	SCS Runoff	33.87	3	732	3.692				Area A-2A
5	SCS Runoff	22.19	3	726	1.791				Area A-3A
6	SCS Runoff	3.516	3	726	0.286			6-30590 + 25-3031	Area A-3B
7	SCS Runoff	15.22	3	726	1.256				Area B-1
8	SCS Runoff	1.884	3	726	0.152				Area B-2
9	SCS Runoff	15.37	3	726	1.249				Area C-1
10	SCS Runoff	14.00	3	726	1.138				Area C-2
11	SCS Runoff	5.471	3	726	0.469				Area D-1
12	Combine	54.45	3	726	5.768	4, 5, 6,			River Park Combined to Saw Mill
13	Combine	20.12	3	726	1.726	1, 2, 3,			River Park Combined to Combined
14	Combine	74.58	3	726	7.494	1, 2, 3, 4,	5, 6,		Total River Park Center
15	Combine	17.11	3	726	1.408	7, 8,			Total Government Center
16	Combine	29.38	3	726	2.387	9, 10,			Total Cacace Center
17	Combine	71.71	3	726	7.169	4, 5, 6, 8,	9,		Existing Combined to SMR
18	Combine	54.82	3	726	4.589	1, 2, 3, 7,	10, 11 , -		Total Existing to Combined Sewer
	v Mill River F	I F ·	tine O	المالي		Davis de 400	N	Tuesday, [

Hydraflow Hydrographs by Intelisolve v9.01

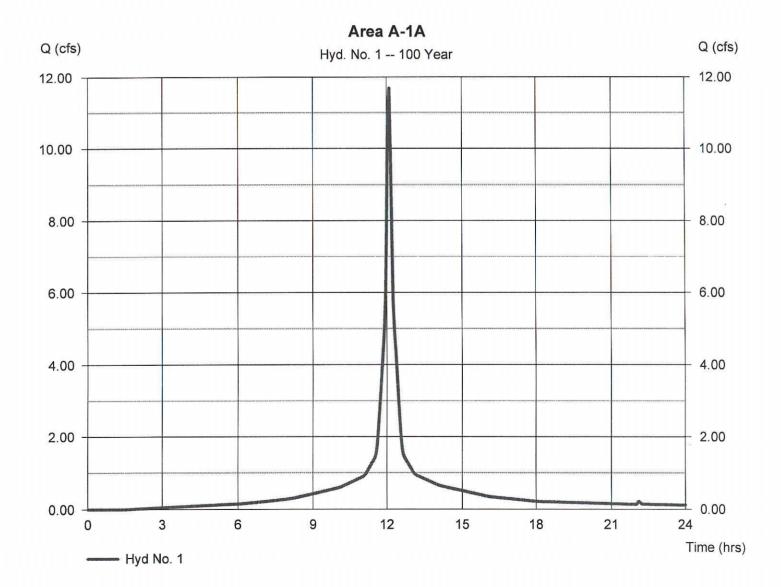
Tuesday, Dec 4, 2007

Hyd. No. 1

Area A-1A

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 3 min = 1.860 ac Drainage area Basin Slope = 0.0 %Tc method = USER = 7.50 inTotal precip. = 24 hrs Storm duration

Peak discharge = 11.70 cfs
Time to peak = 12.10 hrs
Hyd. volume = 1.003 acft
Curve number = 95
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.50 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

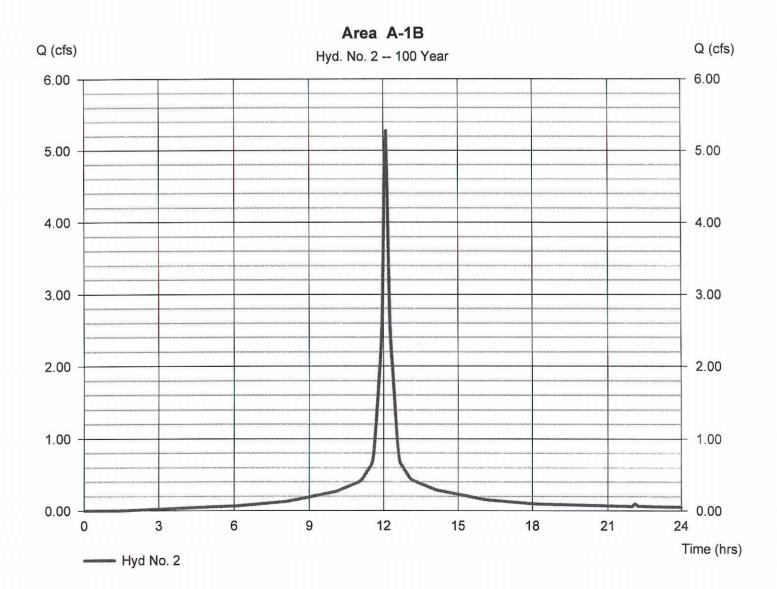
Tuesday, Dec 4, 2007

Hyd. No. 2

Area A-1B

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 3 min
Drainage area = 0.840 ac
Basin Slope = 0.0 %
Tc method = USER
Total precip. = 7.50 in
Storm duration = 24 hrs

Peak discharge = 5.282 cfs
Time to peak = 12.10 hrs
Hyd. volume = 0.453 acft
Curve number = 95
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

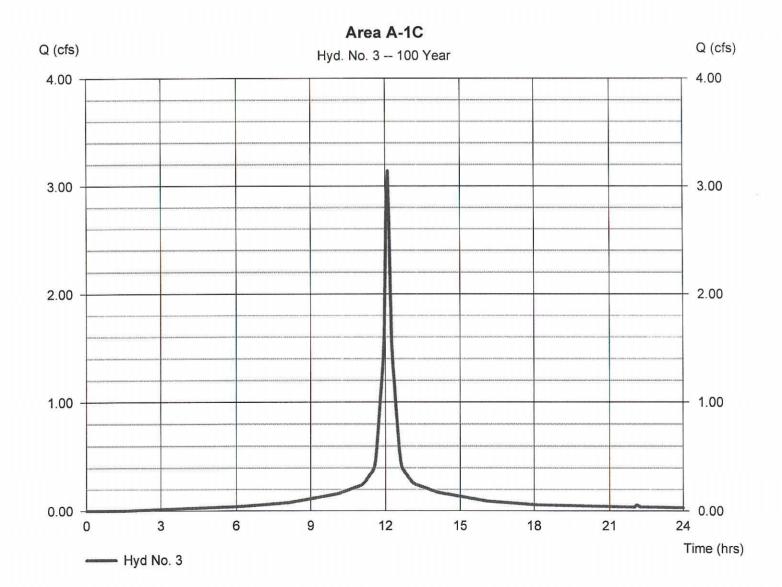
Tuesday, Dec 4, 2007

Hyd. No. 3

Area A-1C

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 3 min
Drainage area = 0.500 ac
Basin Slope = 0.0 %
Tc method = USER
Total precip. = 7.50 in
Storm duration = 24 hrs

Peak discharge = 3.144 cfs
Time to peak = 12.10 hrs
Hyd. volume = 0.270 acft
Curve number = 95
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 4

Storm duration

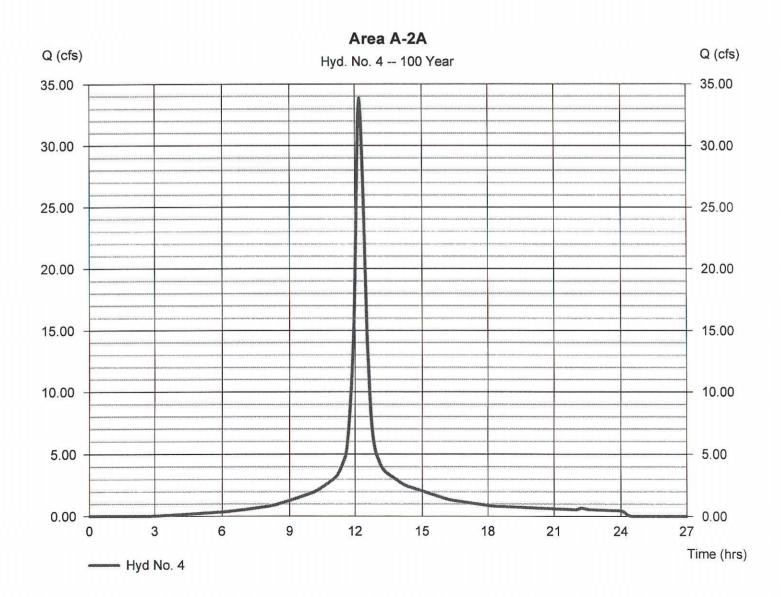
Area A-2A

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 3 min
Drainage area = 6.680 ac
Basin Slope = 0.0 %
Tc method = USER
Total precip. = 7.50 in

= 24 hrs

Peak discharge = 33.87 cfs
Time to peak = 12.20 hrs
Hyd. volume = 3.692 acft
Curve number = 91
Hydraulic length = 0 ft

Time of conc. (Tc) = 19.20 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

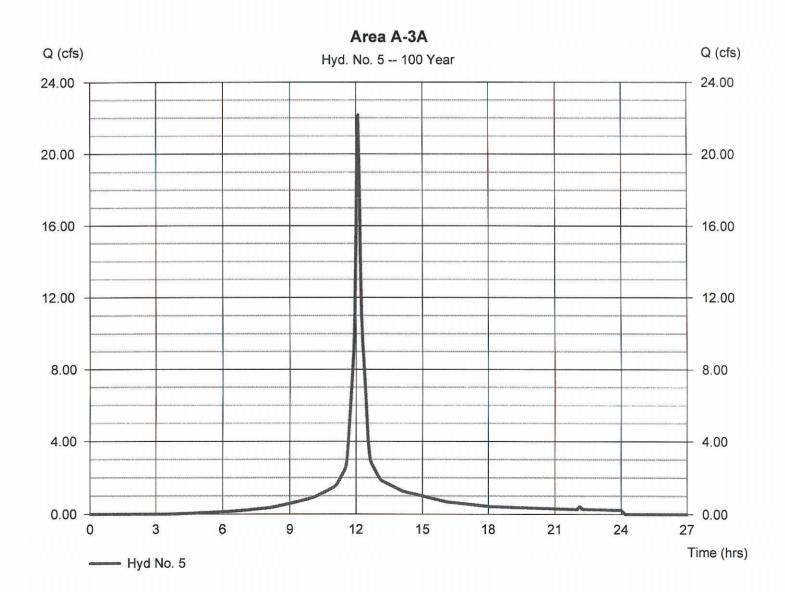
Tuesday, Dec 4, 2007

Hyd. No. 5

Area A-3A

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 3 min
Drainage area = 3.770 ac
Basin Slope = 0.0 %
Tc method = USER
Total precip. = 7.50 in
Storm duration = 24 hrs

Peak discharge = 22.19 cfs
Time to peak = 12.10 hrs
Hyd. volume = 1.791 acft
Curve number = 88
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

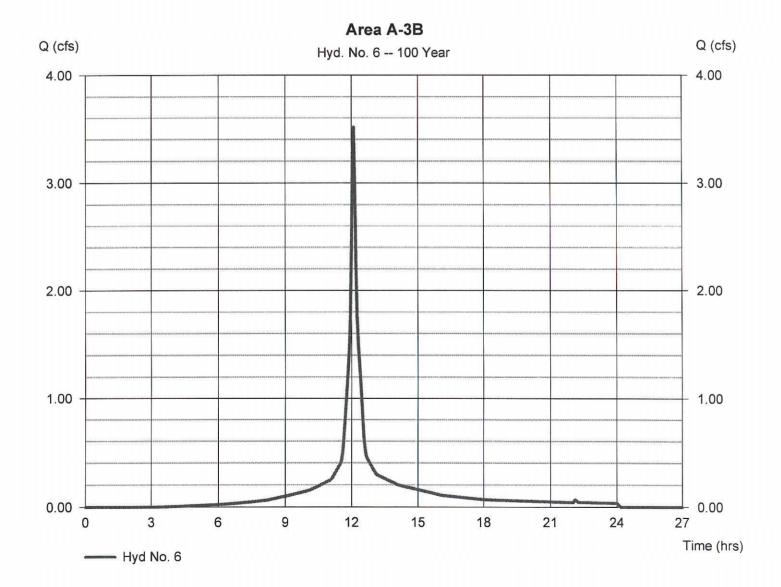
Tuesday, Dec 4, 2007

Hyd. No. 6

Area A-3B

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 3 min
Drainage area = 0.590 ac
Basin Slope = 0.0 %
Tc method = USER
Total precip. = 7.50 in
Storm duration = 24 hrs

Peak discharge = 3.516 cfs
Time to peak = 12.10 hrs
Hyd. volume = 0.286 acft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 7.10 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

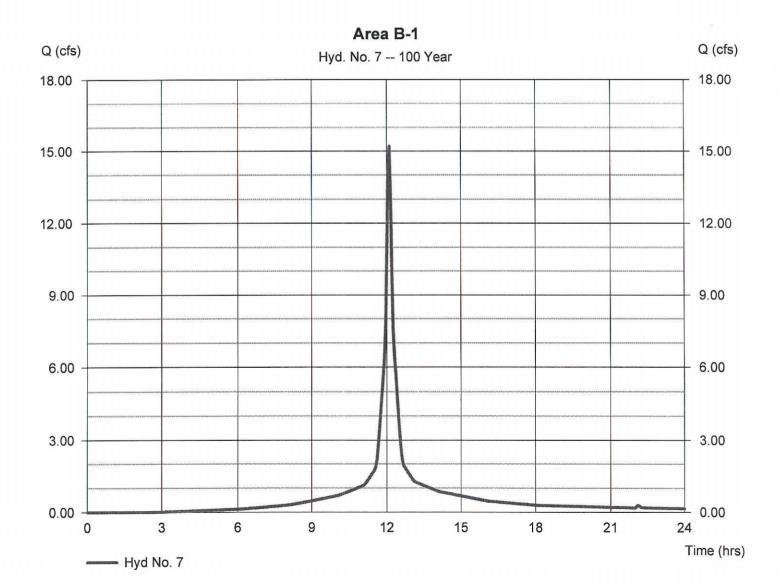
Tuesday, Dec 4, 2007

Hyd. No. 7

Area B-1

= SCS Runoff Hydrograph type Storm frequency = 100 yrs $= 3 \min$ Time interval Drainage area = 2.500 acBasin Slope = 0.0 %Tc method = USER = 7.50 inTotal precip. Storm duration = 24 hrs

Peak discharge = 15.22 cfs
Time to peak = 12.10 hrs
Hyd. volume = 1.256 acft
Curve number = 91
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

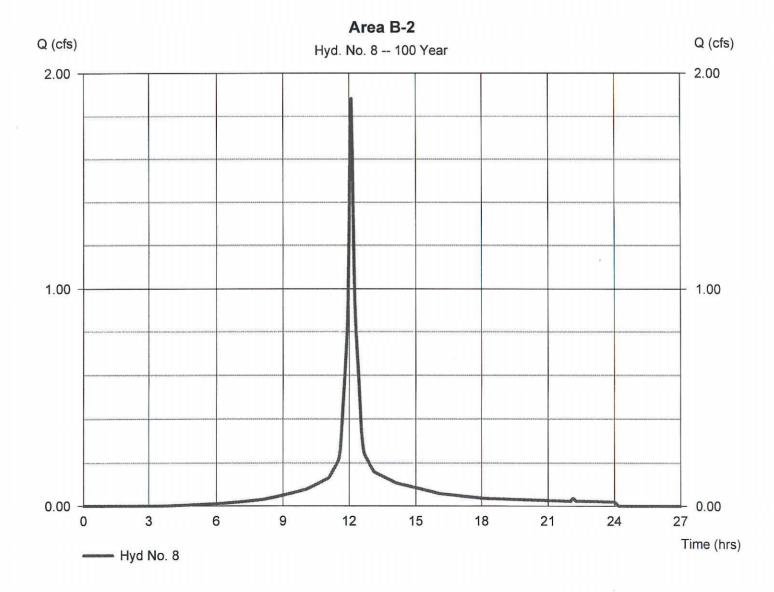
Hyd. No. 8

Area B-2

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval $= 3 \min$ Drainage area = 0.320 acBasin Slope = 0.0 %Tc method = USER = 7.50 inTotal precip. Storm duration = 24 hrs

Peak discharge = 1.884 cfs
Time to peak = 12.10 hrs
Hyd. volume = 0.152 acft
Curve number = 88
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min

Distribution = Type III Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

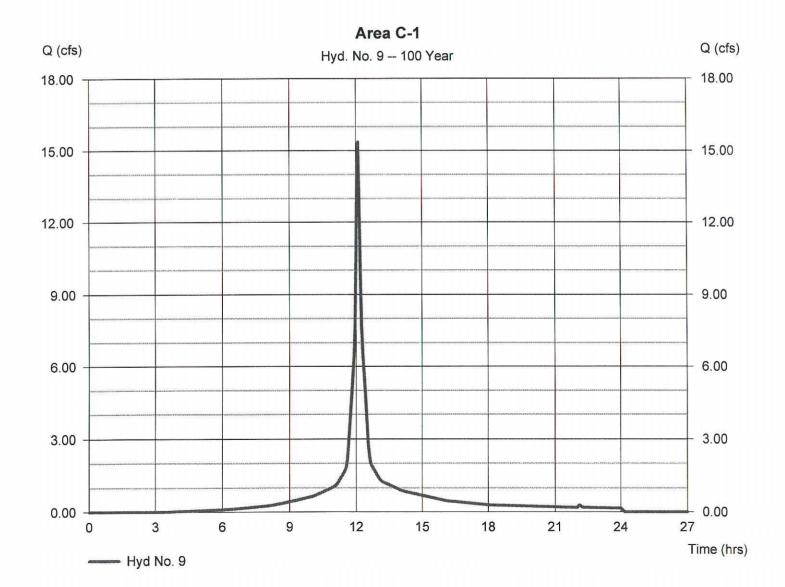
Tuesday, Dec 4, 2007

Hyd. No. 9

Area C-1

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval $= 3 \min$ Drainage area = 2.580 acBasin Slope = 0.0 %Tc method = USER = 7.50 inTotal precip. Storm duration = 24 hrs

Peak discharge = 15.37 cfs
Time to peak = 12.10 hrs
Hyd. volume = 1.249 acft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 10

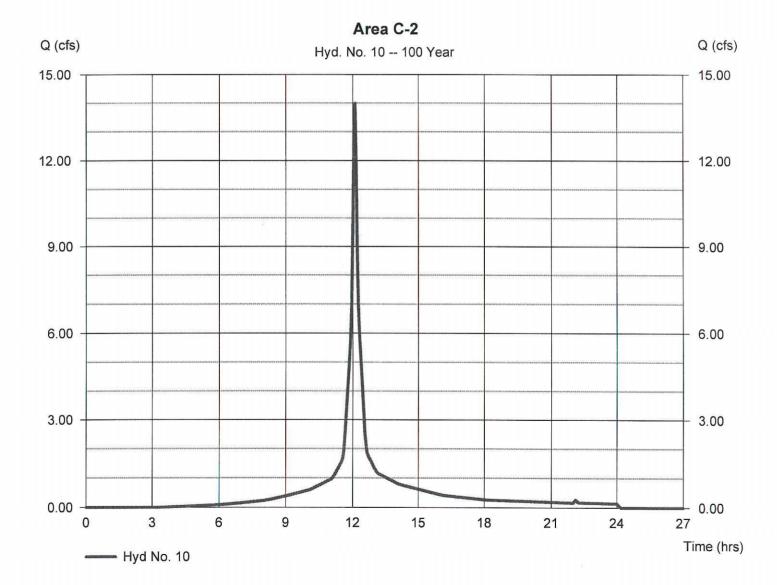
Area C-2

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval $= 3 \min$ Drainage area = 2.350 acBasin Slope = 0.0 %Tc method = USER Total precip. = 7.50 inStorm duration = 24 hrs

Peak discharge = 14.00 cfs
Time to peak = 12.10 hrs
Hyd. volume = 1.138 acft
Curve number = 89
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III

= 484

Shape factor



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

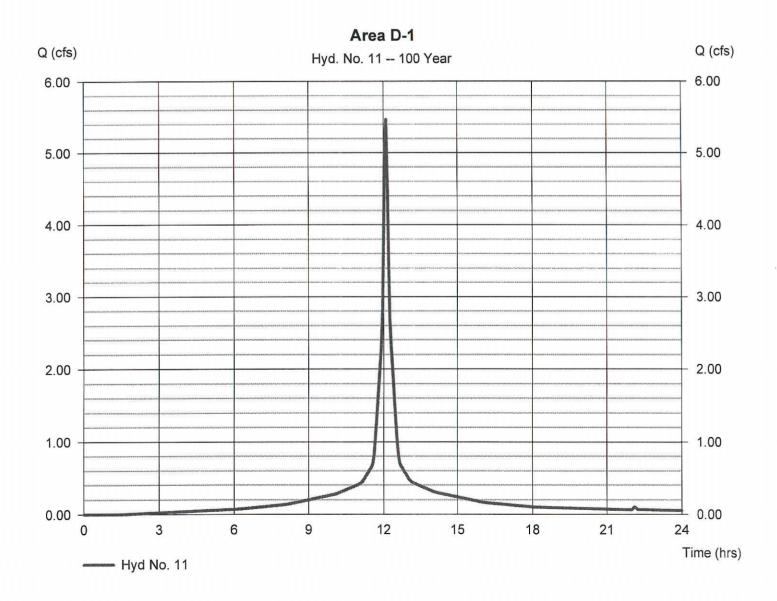
Hyd. No. 11

Area D-1

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval $= 3 \min$ = 0.870 acDrainage area = 0.0 % Basin Slope Tc method = USER Total precip. = 7.50 inStorm duration = 24 hrs

Peak discharge = 5.471 cfs
Time to peak = 12.10 hrs
Hyd. volume = 0.469 acft
Curve number = 95
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min

Distribution = Type III Shape factor = 484



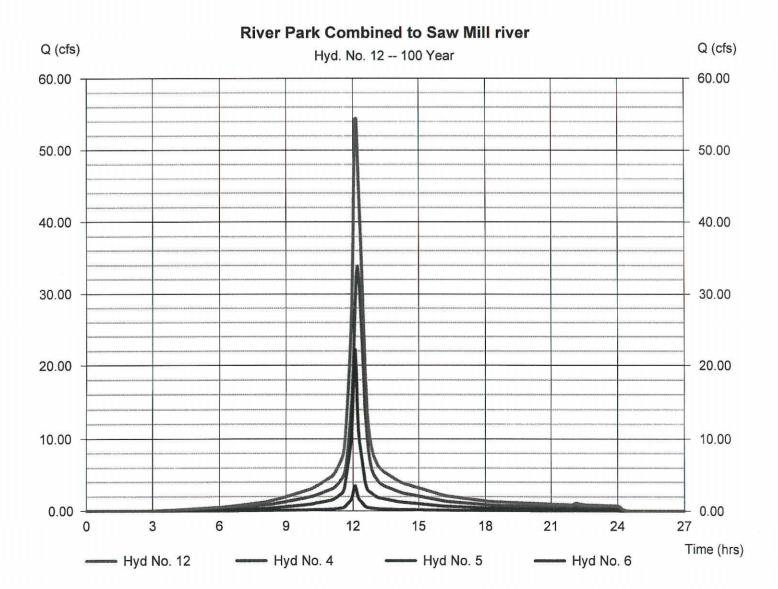
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 12

River Park Combined to Saw Mill river

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 4, 5, 6 Peak discharge = 54.45 cfs Time to peak = 12.10 hrs Hyd. volume = 5.768 acft Contrib. drain. areæ 11.040 ac



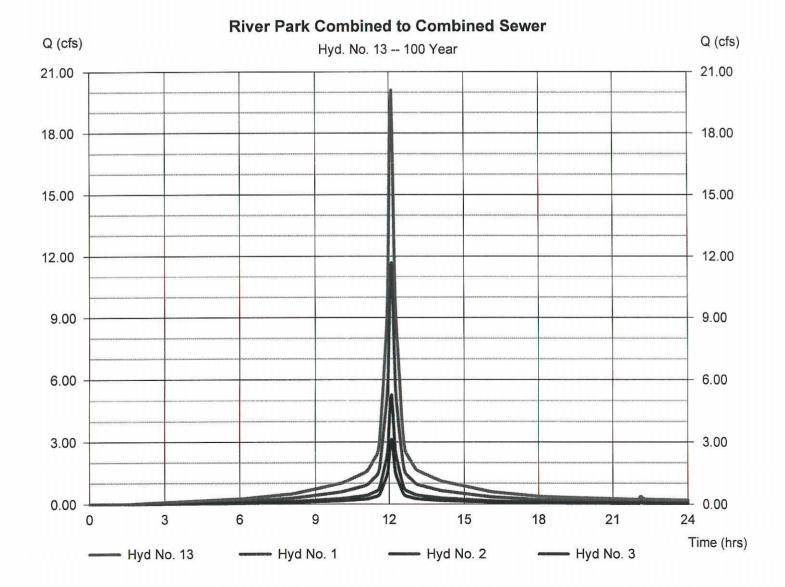
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 13

River Park Combined to Combined Sewer

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 1, 2, 3 Peak discharge = 20.12 cfs Time to peak = 12.10 hrs Hyd. volume = 1.726 acft Contrib. drain. areæ 3.200 ac



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

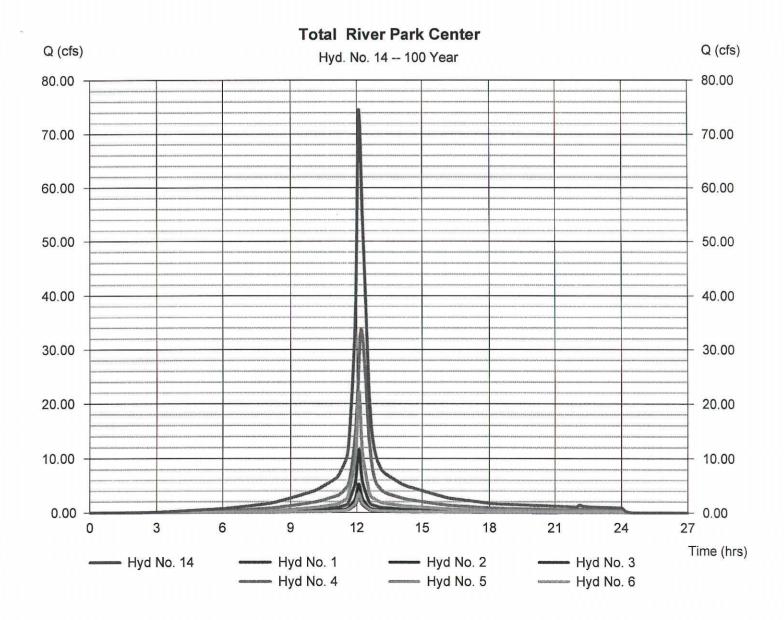
Hyd. No. 14

Total River Park Center

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min

Inflow hyds. = 1, 2, 3, 4, 5, 6

Peak discharge = 74.58 cfs Time to peak = 12.10 hrs Hyd. volume = 7.494 acft Contrib. drain. areæ 14.240 ac



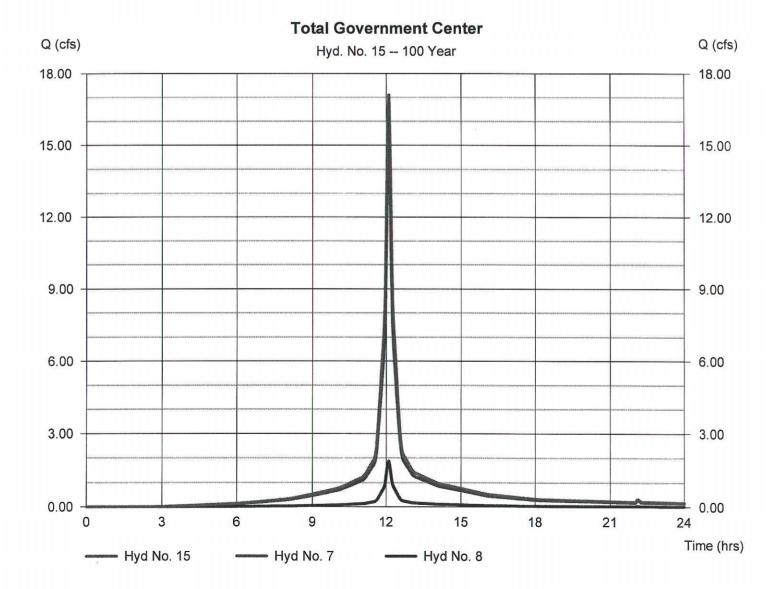
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 15

Total Government Center

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 7, 8 Peak discharge = 17.11 cfs Time to peak = 12.10 hrs Hyd. volume = 1.408 acft Contrib. drain. areæ= 2.820 ac



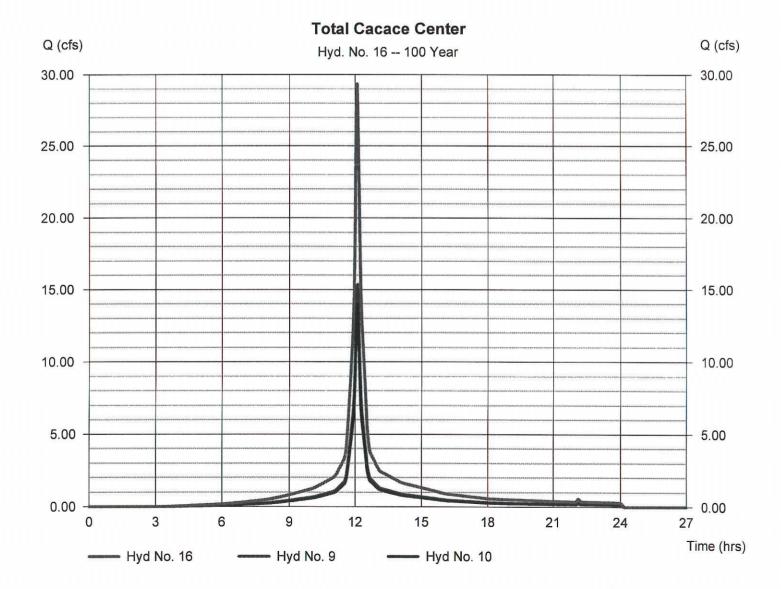
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 16

Total Cacace Center

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 9, 10 Peak discharge = 29.38 cfs Time to peak = 12.10 hrs Hyd. volume = 2.387 acft Contrib. drain. areæ 4.930 ac



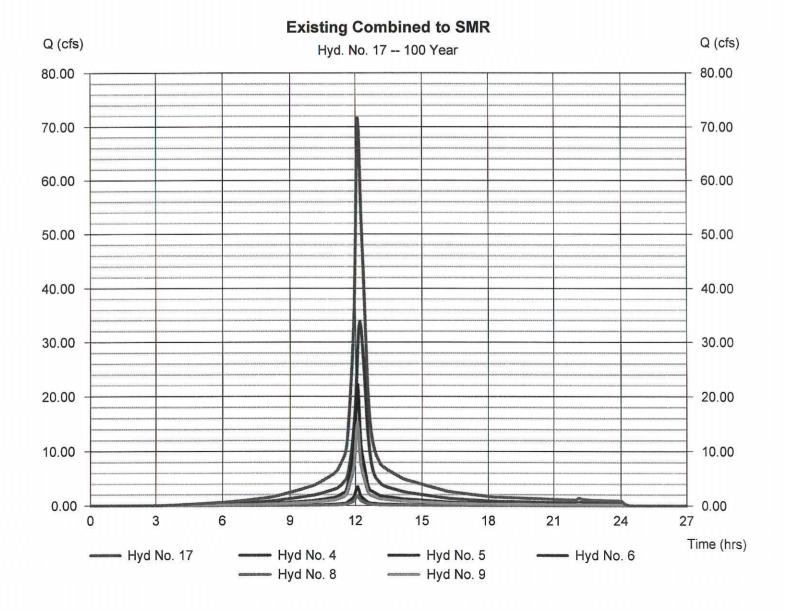
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 17

Existing Combined to SMR

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 4, 5, 6, 8, 9 Peak discharge = 71.71 cfs Time to peak = 12.10 hrs Hyd. volume = 7.169 acft Contrib. drain. areæ 13.940 ac



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

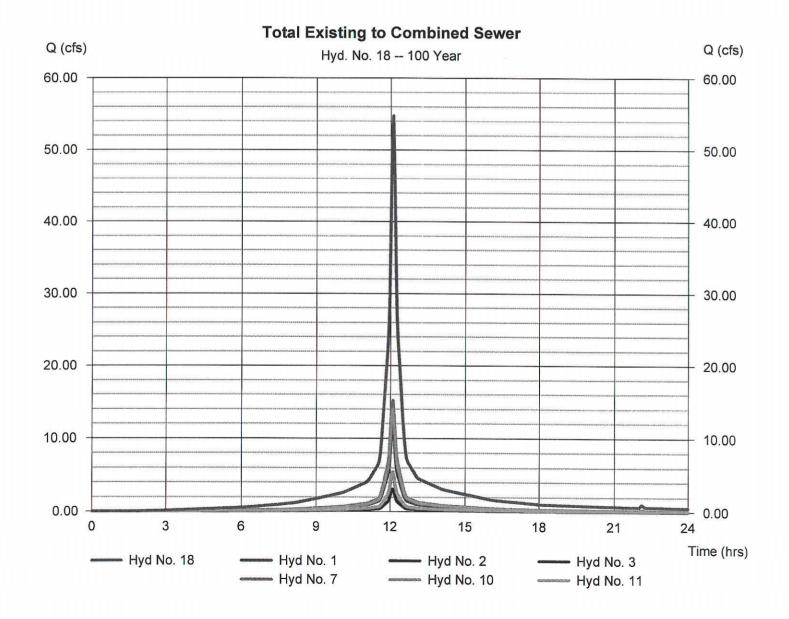
Hyd. No. 18

Total Existing to Combined Sewer

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min

Inflow hyds. = 1, 2, 3, 7, 10, 11

Peak discharge = 54.82 cfs Time to peak = 12.10 hrs Hyd. volume = 4.589 acft Contrib. drain. areæ 8.920 ac



WinTR-55 Current Data Description

--- Identification Data ---

User:

B. SANTOS

Date: 11/28/2007 Units: English

Project: MEG 106100.03

SubTitle: EXIST COND - Larkin Plaza

Areal Units: Acres

State: New York County: Westchester

Filename: P:\Proj106\106100.03\8. Technical (Rpts, Calcs, Specs)\Calculations\Computer Analysis\Wi

--- Sub-Area Data ---

Name Description		Reach	Area(ac)	RCN	TC
PLaza	Parking and Plaza	Outlet	1.89	94	0.353

Total area: 1.89 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	1-Yr
(in)	(in)	(in)	(in)	(in)	(in)	(in)
3.5	4.5	5.0	6.0	7.0	7.5	2.8

Storm Data Source:

Westchester County, NY (NRCS)

Rainfall Distribution Type:

Type III Dimensionless Unit Hydrograph: <standard>



Natural Resources Conservation Service

11/26/2007 Page 1 of 3

Web Soil Survey 2.0 National Cooperative Soil Survey

Map Unit Legend

Westchester County, New York (NY119)							
Map Unit Sy	mbol	Map Unit Name	10.4%	Acres in AOI		Percent of AOI	
Uf Urban land					1.9	100.0%	
Totals for Area of In	nterest (AOI)				1.9	100.0%	

MEG 106100.03 EXIST COND - Larkin Plaza Westchester County, New York

Storm Data

Rainfall Depth by Rainfall Return Period

2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	1-Yr
(in)	(in)	(in)	(in)	(in)	(in)	(in)
3.5	4.5	5.0	6.0	7.0	7.5	2.8

Westchester County, NY (NRCS)

Storm Data Source: Westchester Rainfall Distribution Type: Type III Dimensionless Unit Hydrograph: <standard>

MEG 106100.03 EXIST COND - Larkin Plaza Westchester County, New York

Watershed Peak Table

Sub-Area or Reach Identifier	Peak 2-Yr (cfs)	Flow by 10-Yr (cfs)	Rainfall 25-Yr (cfs)	Return Period 100-Yr (cfs)	
SUBAREAS PLaza	3.97	5.90	7.17	9.07	
REACHES					
OUTLET	3.97	5.90	7.17	9.07	

MEG 106100.03 EXIST COND - Larkin Plaza Westchester County, New York

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
PLaza	1.89	0.353	94	Outlet	Parking and Plaza

Total Area: 1.89 (ac)

MEG 106100.03 EXIST COND - Larkin Plaza Westchester County, New York

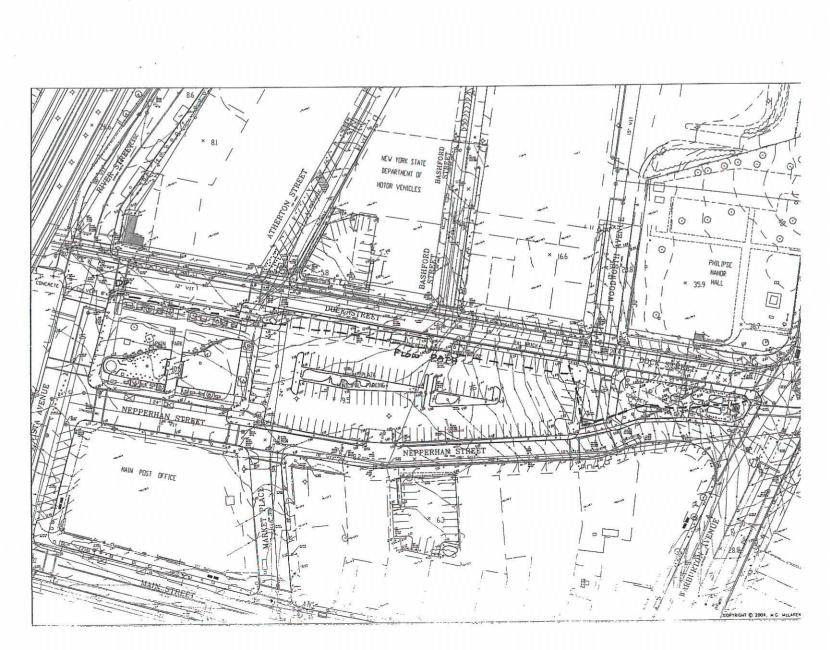
Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
PLaza							
SHEET	99	0.0200	0.240				0.226
SHALLOW	30	0.0200	0.025				0.003
CHANNEL	88					2.000	0.012
CHANNEL	603					1.500	0.112
				Ti	me of Conce	ntration	0.353

MEG 106100.03 EXIST COND - Larkin Plaza Westchester County, New York

Sub-Area Land Use and Curve Number Details

Sub-Ar Identif	7 7 7	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
PLaza	Open space; grass cover 50% to 75% (fair) C	.372	79
	Paved parking lots, roofs, driveways	С	1.522	98
	Total Area / Weighted Curve Number		1.89	94
	* 7			==



APPENDIX B PROPOSED CONDITIONS

WinTR-55 Current Data Description

--- Identification Data ---

User: B. SANTOS Project: MEG 106100

Date:

2/23/2007

Units:

English

Subtitle: PROP COND RUN-1 : RIVER PARK CENTER PROJECT

Areal Units: Acres

State: New York County: Westchester

Filename: P:\Projl06\106100\8. Technical (Rpts, Calcs, Specs)\Calculations\Computer Analysis\Storm

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
A-1A A-1C A-2A B-1 C-1	R-PARK TO C-SEWER R-PARK TO C-SEWER R-PARK TO STORM G=CENTER TO STORM D CACASE CENTER TO STORM CACASE CENTER TO C-SEWER	Outlet Outlet Outlet Outlet Outlet	0.3 0.41 13.29 2.82 3.09 1.84	98 98 94 96 98	0.100 0.100 0.100 0.100 0.100

Total area: 21.75 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	1-Yr
(in)	(in)	(in)	(in)	(in)	(in)	(in)
3.5	4.5	5.0	6.0	7.0	7.5	2.8

Storm Data Source:

Westchester County, NY (NRCS)

Rainfall Distribution Type: Type III Dimensionless Unit Hydrograph: <standard>

MEG 106100 PROP COND RUN-1: RIVER PARK CENTER PROJECT Westchester County, New York

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
A-1A	.30	0.100	98	Outlet	R-PARK TO C-SEWER
A-1C	.41	0.100	98	Outlet	R-PARK TO C-SEWER
A-2A	13.29	0.100	94	Outlet	R-PARK TO STORM
B-1	2.82	0.100	96	Outlet	G=CENTER TO STORM D
C-1	3.09	0.100	98	Outlet	CACASE CENTER TO STORM
C-2	1.84	0.100	87	Outlet	CACASE CENTER TO C-SEWER

Total Area: 21.75 (ac)

MEG 106100 PROP COND RUN-1 : RIVER PARK CENTER PROJECT Westchester County, New York

Sub-Area Time of Concentration Details

Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
A-1A User-proví							0.100
				Tip	me of Conce		0.100
A-1C User-provid	ded						0.100
				Ti	ne of Conce		0.100
A-2A User-provid	ded						0.100
				Tir	ne of Conce		0.100
B-1 User-provid	ied						0.100
				Tir	me of Conce		0.100
C-1 User-provid	ied						0.100
-				Tir	ne of Conce		0.100
C-2 User-provid	ied						0.100
				Tir	ne of Conce		0.100

MEG 106100 PROP COND RUN-1: RIVER PARK CENTER PROJECT Westchester County, New York

Sub-Area Land Use and Curve Number Details

Sub-Area Identifie		Hydrologic Soil Group	Area (ac)	Curve Number
		מ	.3	98
	Total Area / Weighted Curve Number		.3	98 ==
A-1C	Paved parking lots, roofs, driveways	D	.41	98
	Total Area / Weighted Curve Number		.41	98 ==
A-2A	Open space; grass cover > 75% {good Paved parking lots, roofs, driveways	D D	2.96 10.33	80 98
	Total Area / Weighted Curve Number		13.29	94
B-1	Open space; grass cover > 75% (good) Paved parking lots, roofs, driveways	D D	.31 2.51	80 98
	Total Area / Weighted Curve Number		2.82	96 ==
C-1	Paved parking lots, roofs, driveways Paved; curbs and storm sewers	D D	.8 2.29	98 98
	Total Area / Weighted Curve Number		3.09	98 ==
C-2	Open space; grass cover > 75% {good} Paved parking lots, roofs, driveways	D D	1.1	80 98
	Total Area / Weighted Curve Number		1.84	87 ==

MEG 106100 PROPOSED CONDITION RUN-2 : PALISADE AVE SITE Westchester County, New York

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)		Receiving Reach	Sub-Area Description
D-2	.87	0.100	96	Outlet	R-PRK GAR TO RIVER

Total Area: .87 (ac)

MEG 106100 PROPOSED CONDITION RUN-2 : PALISADE AVE SITE Westchester County, New York

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wette Perime (ft)		Travel Time (hr)
D-2 SHEET SHALLOW SHALLOW CHANNEL	50 100 30 120	0.0100 0.0400 0.0900	0.011 0.025 0.050			2.000	0.015 0.007 0.002 0.017
				Ti	me of C	oncentration	0.100

MEG 106100 PROPOSED CONDITION RUN-2 : PALISADE AVE SITE Westchester County, New York

Sub-Area Land Use and Curve Number Details

Sub-Are Identif		Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
D-2	Open space; grass cover > 75% (good Paved parking lots, roofs, driveways) D D	.09 .78	80 98
	Total Area / Weighted Curve Number		.87	96 ==

Proj. file: Saw Mill River Flow Prop Conditions 12-4-07.gpw

lydrograph	Inflow				Hydrograph					
type (origin)	Hyd(s)	1-Yr	2-Yr	3-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	description
SCS Runoff			0.884			1.270	1.527	2234-	1.912	Area A-1A
SCS Runoff			1.209			1.736	2.087		2.613	Area A-1C
CS Runoff			36.50			54.09	65.71		83.03	Area A-2A
CS Runoff		*****	8.075			11.75	14.19	100 half star arts and man and	17.83	Area B-1
SCS Runoff			9.110		***************************************	13.09	15.73		19.69	Area C-1
CS Runoff			4.088			6.569	8.224		10.69	Area C-2
CS Runoff			2.443			3.587	4.343		5.471	Area D-1
Combine	1, 2, 3,		38.59			57.10	69.32		87.55	Total river Park Center Prop.
Combine	5, 6,		13.20		per 100 100 100 100 100 100 100	19.65	23.95		30.38	total Cacace Center Prop
Combine	1, 2,		2.093			3.007	3.614		4.524	River Park Combined to Combined
Combine	3, 4, 5, 7,		56.13			82.52	99.97		126.02	Prop Combined to SMR
Combine	1, 2, 6,	M. 60 TO TO TO 14	6.181			9.575	11.84		15.22	Total Prop to Combined Sewer
CS Runoff			513.89		*******	789.40	971.62		1242.88	Yonkers Area to Saw Mill river
Combine	11, 13		549.95			842.15	1035.41		1323.16	Total Prop to SMR minus Upstrea

Tuesday, Dec 4, 2007

	Hydraflow Hydrographs by Intellsolve v9.0								
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph description
1	SCS Runoff	0.884	3	726	0.077				Area A-1A
2	SCS Runoff	1.209	3	726	0.105			Participation in	Area A-1C
3	SCS Runoff	36.50	3	726	2.944				Area A-2A
4	SCS Runoff	8.075	3	726	0.671				Area B-1
5	SCS Runoff	9.110	3	726	0.789			<u></u>	Area C-1
6	SCS Runoff	4.088	3	726	0.314				Area C-2
7	SCS Runoff	2.443	3	726	0.200		en vermen en en		Area D-1
8	Combine	38.59	3	726	3.125	1, 2, 3,		m = ***********************************	Total river Park Center Prop.
9	Combine	13.20	3	726	1.102	5, 6,		Promise Pro-en-en-	total Cacace Center Prop
10	Combine	2.093	3	726	0.181	1, 2,			River Park Combined to Combined
11	Combine	56.13	3	726	4.603	3, 4, 5, 7,			Prop Combined to SMR
12	Combine	6.181	3	726	0.495	1, 2, 6,			Total Prop to Combined Sewer
13	SCS Runoff	513.89	3	732	53.524				Yonkers Area to Saw Mill river
14	Combine	549.95	3	732	58.128	11, 13		Middle Life and wat	Total Prop to SMR minus Upstrea
Sav	v Mill River F	low Prop	Conditi	ons 12-4-	OR ejbu n P	Period: 2 Yo	ear	Tuesday, [Dec 4, 2007

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph description
1	SCS Runoff	1.270	3	726	0.112				Area A-1A
2	SCS Runoff	1.736	3	726	0.153		***************************************		Area A-1C
3	SCS Runoff	54.09	3	726	4.473		PHYSONA		Area A-2A
4	SCS Runoff	11.75	3	726	0.999				Area B-1
5	SCS Runoff	13.09	3	726	1.150				Area C-1
6	SCS Runoff	6.569	3	726	0.513				Area C-2
7	SCS Runoff	3.587	3	726	0.300			PERMIN	Area D-1
8	Combine	57.10	3	726	4.737	1, 2, 3,			Total river Park Center Prop.
9	Combine	19.65	3	726	1.663	5, 6,	No Vel de las sus		total Cacace Center Prop
10	Combine	3.007	3	726	0.264	1, 2,		WD11	River Park Combined to Combine
11	Combine	82.52	3	726	6.922	3, 4, 5, 7,			Prop Combined to SMR
12	Combine	9.575	3	726	0.777	1, 2, 6,			Total Prop to Combined Sewer
13	SCS Runoff	789.40	3	732	83.876	No. 100 and use			Yonkers Area to Saw Mill river
14	Combine	842.15	3	732	90.798	11, 13			Total Prop to SMR minus Upstrea
	;								
Sav	/ Mill River F	low Prop	Conditi	ons 12-4-	O'Refoun P	eriod: 10 \	/ear	Tuesday, [Dec 4 2007

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph description
1	SCS Runoff	1.527	3	726	0.135		e name in er	*****	Area A-1A
2	SCS Runoff	2.087	3	726	0.185				Area A-1C
3	SCS Runoff	65.71	3	726	5.499			77-770	Area A-2A
4	SCS Runoff	14.19	3	726	1.218				Area B-1
5	SCS Runoff	15.73	3	726	1.391				Area C-1
6	SCS Runoff	8.224	3	726	0.649	***			Area C-2
7	SCS Runoff	4.343	3	726	0.368				Area D-1
8	Combine	69.32	3	726	5.819	1, 2, 3,	****		Total river Park Center Prop.
9	Combine	23.95	3	726	2.040	5, 6,	the lab the sale		total Cacace Center Prop
10	Combine	3.614	3	726	0.320	1, 2,	*****		River Park Combined to Combined
11	Combine	99.97	3	726	8.476	3, 4, 5, 7,	*****		Prop Combined to SMR
12	Combine	11.84	3 .	726	0.969	1, 2, 6,	20 00 PG 40 20 AA		Total Prop to Combined Sewer
13	SCS Runoff	971.62	3	732	104.415		20 to 40 to to		Yonkers Area to Saw Mill river
14	Combine	1035.41	3	732	112.891	11, 13			Total Prop to SMR minus Upstrea
Sav	v Mill River F	low Prop	Conditi	ons 12-4-	O'Rejbukn F	Period: 25 \	Year	Tuesday. [Dec 4, 2007

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph description
1	SCS Runoff	1.912	3	726	0.170				Area A-1A
2	SCS Runoff	2.613	3	726	0.233				Area A-1C
3	SCS Runoff	83.03	3	726	7.045				Area A-2A
4	SCS Runoff	17.83	3	726	1.547				Area B-1
5	SCS Runoff	19.69	3	726	1.753			Potent Modern	Area C-1
3	SCS Runoff	10.69	3	726	0.857				Area C-2
7	SCS Runoff	5.471	3	726	0.469	~~~			Area D-1
3	Combine	87.55	3	726	7.447	1, 2, 3,		DE WALLES OF	Total river Park Center Prop.
9	Combine	30.38	3	726	2.610	5, 6,		******	total Cacace Center Prop
10	Combine	4.524	3	726	0.403	1, 2,			River Park Combined to Combined
11	Combine	126.02	3	726	10.814	3, 4, 5, 7,		man, or other	Prop Combined to SMR
12	Combine	15.22	3	726	1.260	1, 2, 6,			Total Prop to Combined Sewer
13	SCS Runoff	1242.88	3	732	135.461				Yonkers Area to Saw Mill river
14	Combine	1323.16	3	732	146.275	11, 13			Total Prop to SMR minus Upstrea
200	v Mill River F	low Prop	Conditi	ons 12-4-	ORentowo F	Period: 100	Year	Tuesday, I	Dec 4, 2007

Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

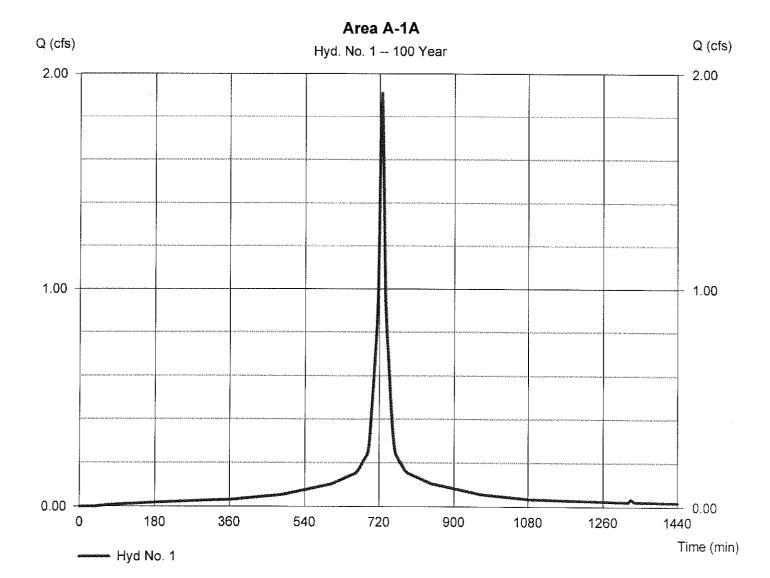
Hyd. No. 1

Area A-1A

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 3 minDrainage area = 0.300 acBasin Slope = 0.0 %Tc method = USER Total precip. = 7.50 inStorm duration = 24 hrs

Peak discharge = 1.912 cfs
Time to peak = 726 min
Hyd. volume = 0.170 acft
Curve number = 98
Hydraulic length = 0 ft

Curve number = 98
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

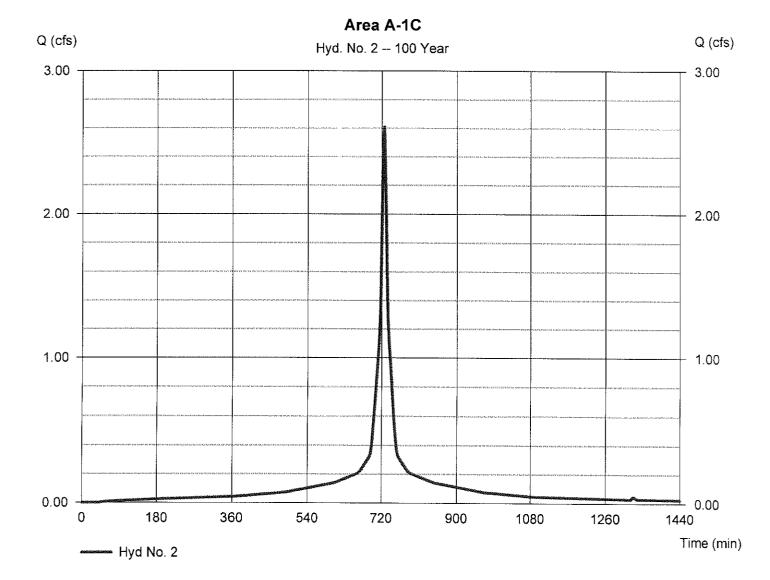
Hyd. No. 2

Area A-1C

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval $= 3 \min$ Drainage area = 0.410 ac= 0.0 % Basin Slope Tc method = USER Total precip. = 7.50 inStorm duration = 24 hrs

Peak discharge = 2.613 cfs
Time to peak = 726 min
Hyd. volume = 0.233 acft
Curve number = 98
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III

Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

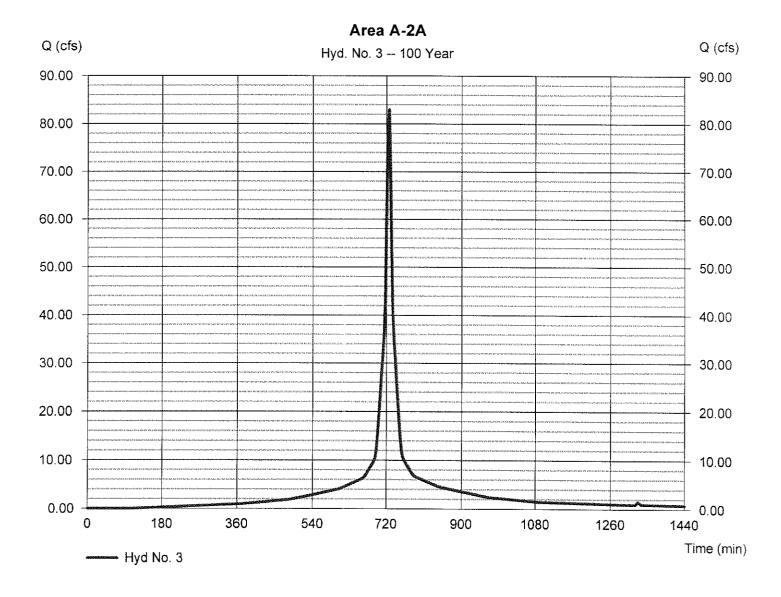
Hyd. No. 3

Area A-2A

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 3 min Drainage area = 13.290 ac = 0.0 %Basin Slope Tc method = USER Total precip. = 7.50 inStorm duration = 24 hrs

Peak discharge = 83.03 cfs
Time to peak = 726 min
Hyd. volume = 7.045 acft
Curve number = 94
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min

Distribution = Type III Shape factor = 484



Hydrafiow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 4

Area B-1

Hydrograph type = SCS Runoff
Storm frequency = 100 yrs
Time interval = 3 min
Drainage area = 2.820 ac
Basin Slope = 0.0 %
Tc method = USER
Total precip. = 7.50 in
Storm duration = 24 hrs

Peak discharge = 17.83 cfs
Time to peak = 726 min
Hyd. volume = 1.547 acft

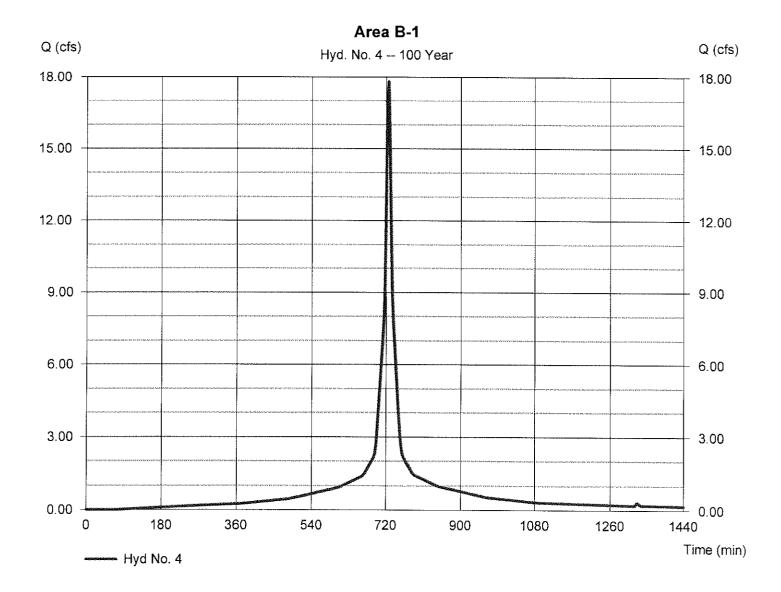
Curve number = 96

Hydraulic length = 0 ft

Time of conc. (Tc) = 6.00 min

Distribution = Type III

Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 5

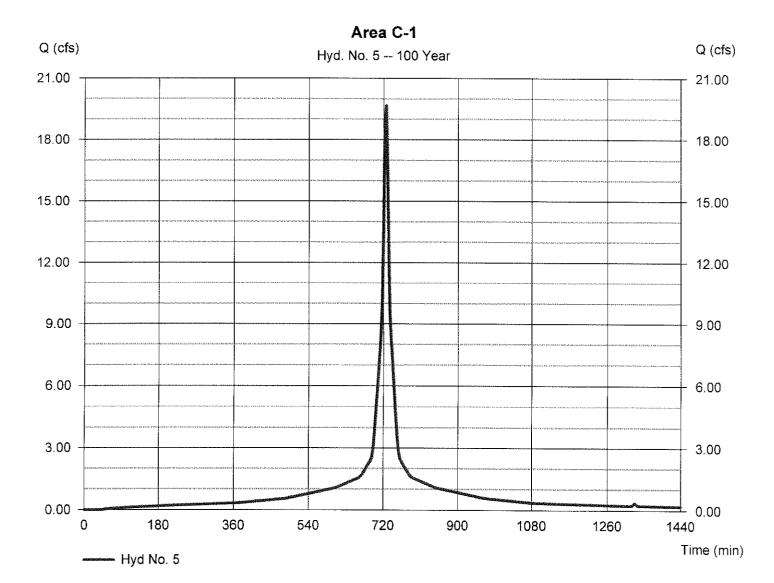
Area C-1

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval $= 3 \min$ Drainage area = 3.090 acBasin Slope = 0.0 % Tc method = USER Total precip. = 7.50 inStorm duration = 24 hrs

Peak discharge = 19.69 cfs
Time to peak = 726 min
Hyd. volume = 1.753 acft
Curve number = 98
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III

= 484

Shape factor



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

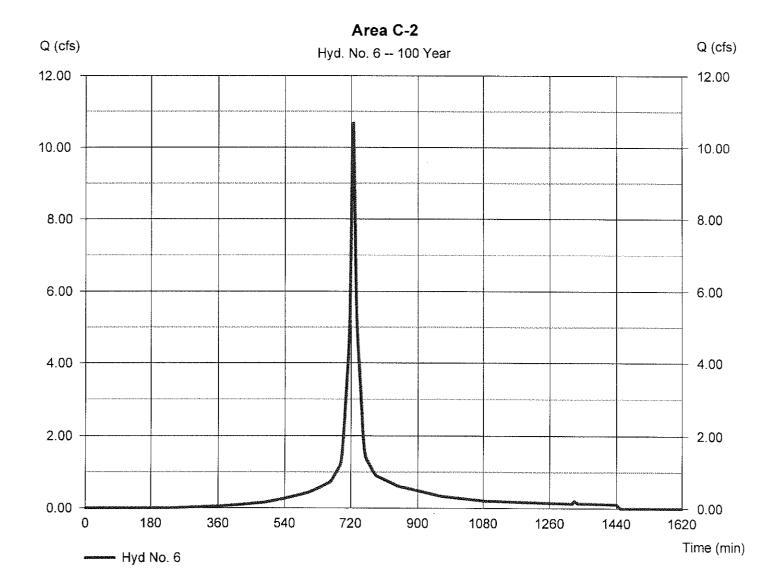
Hyd. No. 6

Area C-2

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 3 min Drainage area = 1.840 ac Basin Slope = 0.0 % Tc method = USER Total precip. = 7.50 inStorm duration = 24 hrs

Peak discharge = 10.69 cfs
Time to peak = 726 min
Hyd. volume = 0.857 acft

Curve number = 0.657 act
Curve number = 87
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III
Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

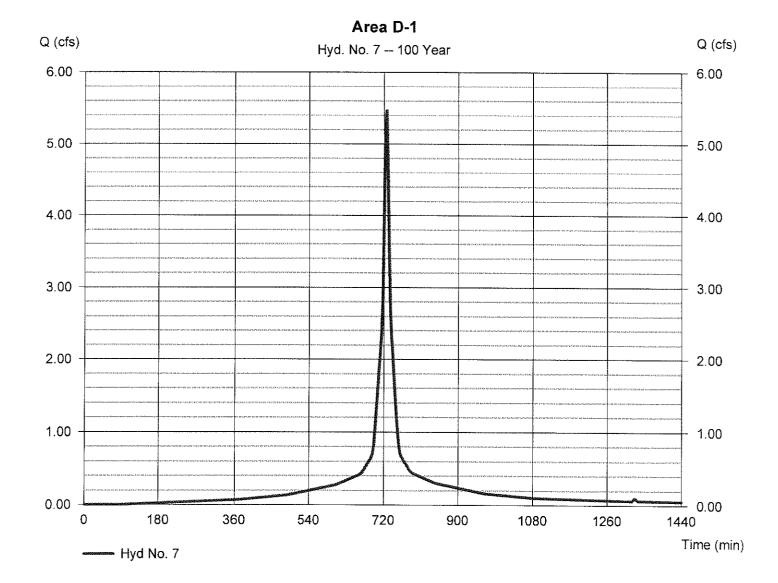
Hyd. No. 7

Area D-1

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 3 min Drainage area = 0.870 acBasin Slope = 0.0 %Tc method = USER Total precip. = 7.50 inStorm duration = 24 hrs

Peak discharge = 5.471 cfs
Time to peak = 726 min
Hyd. volume = 0.469 acft
Curve number = 95

Curve number = 95
Hydraulic length = 0 ft
Time of conc. (Tc) = 6.00 min
Distribution = Type III
Shape factor = 484



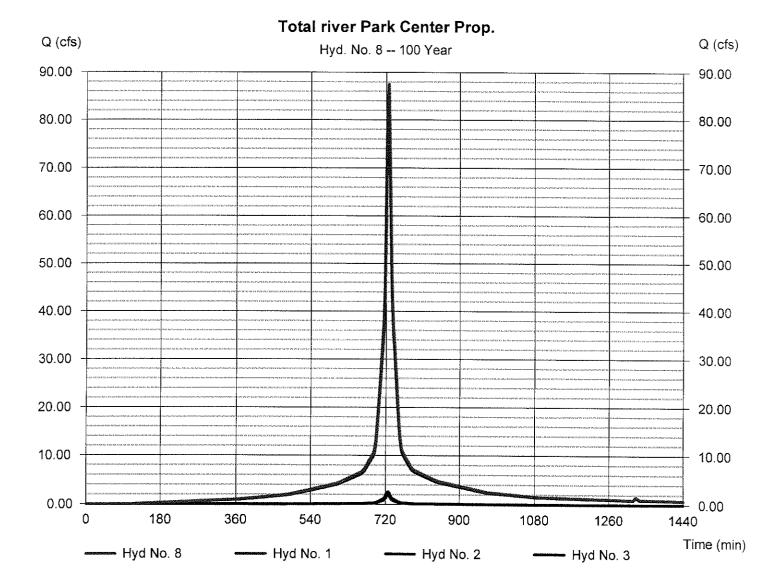
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 8

Total river Park Center Prop.

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 1, 2, 3 Peak discharge = 87.55 cfs Time to peak = 726 min Hyd. volume = 7.447 acft Contrib. drain. areæ 14.000 ac



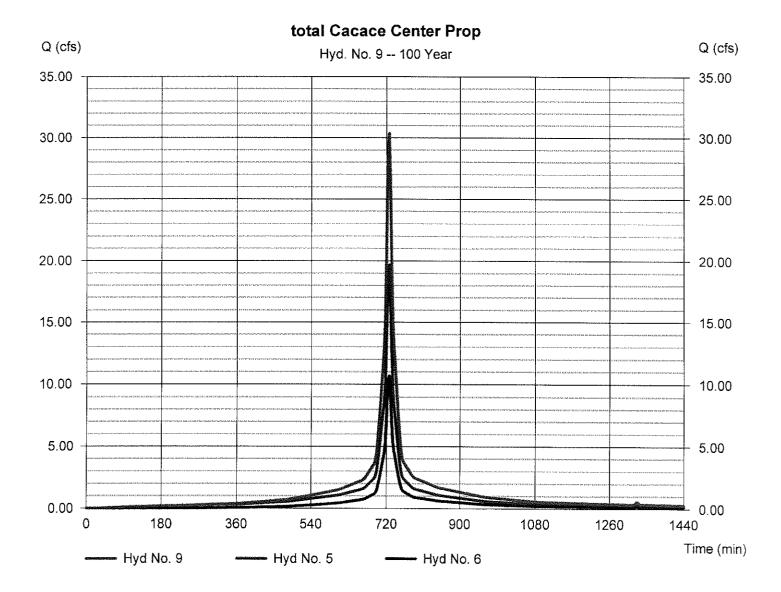
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 9

total Cacace Center Prop

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 5, 6 Peak discharge = 30.38 cfs Time to peak = 726 min Hyd. volume = 2.610 acft Contrib. drain. areæ 4.930 ac



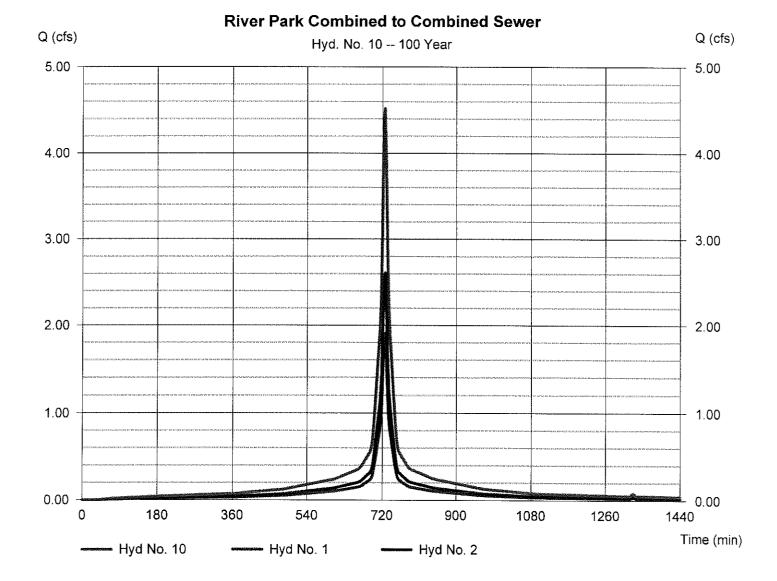
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 10

River Park Combined to Combined Sewer

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 1, 2 Peak discharge = 4.524 cfs Time to peak = 726 min Hyd. volume = 0.403 acft Contrib. drain. areæ 0.710 ac



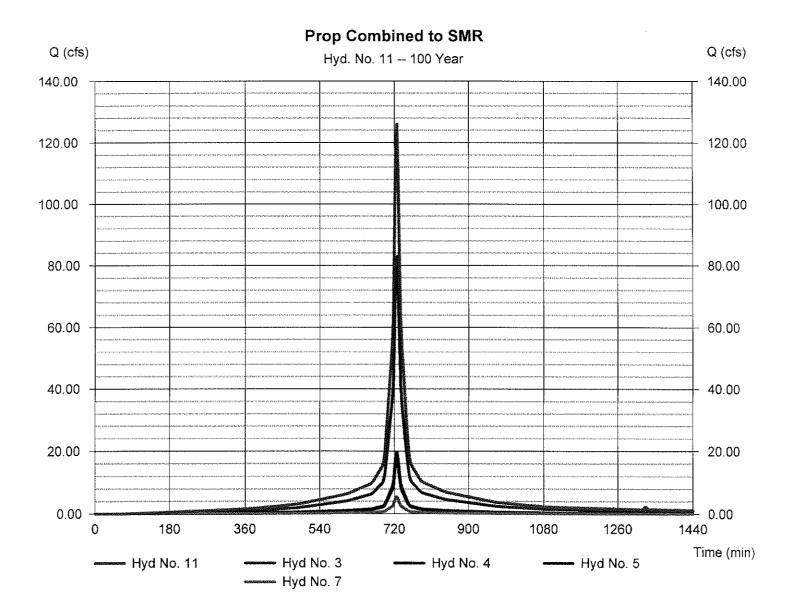
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 11

Prop Combined to SMR

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 3, 4, 5, 7 Peak discharge = 126.02 cfs Time to peak = 726 min Hyd. volume = 10.814 acft Contrib. drain. areæ 20.070 ac



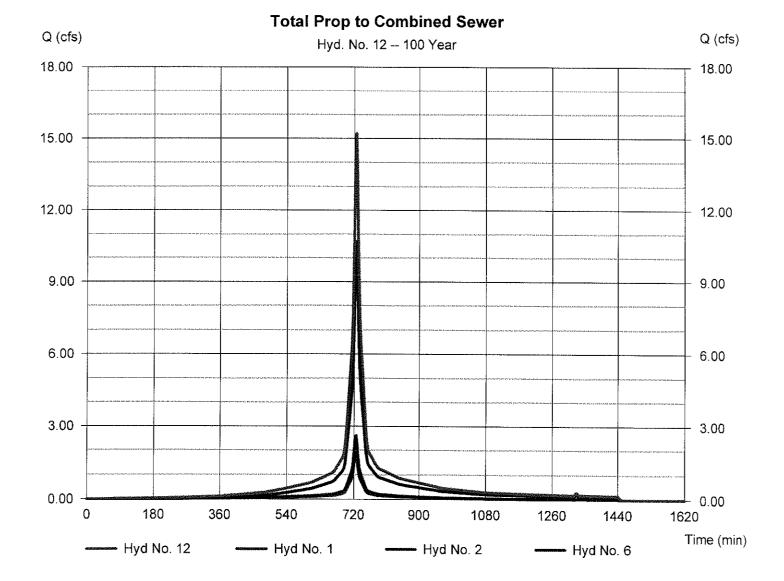
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 12

Total Prop to Combined Sewer

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 1, 2, 6 Peak discharge = 15.22 cfs Time to peak = 726 min Hyd. volume = 1.260 acft Contrib. drain. areæ 2.550 ac



WinTR-55 Current Data Description

--- Identification Data ---

User: B. SANTOS Project: MEG 106100.03 SubTitle: PROP. COND - Larkin Plaza

Date: 11/28/2007 Units: English

Areal Units: Acres

State: New York County: Westchester

Filename: P:\Proj106\106100.03\8. Technical (Rpts, Calcs, Specs)\Calculations\Computer Analysis\Wi

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
Plaza	Park Area and River	Outlet	1.89	84	0.352

Total area: 1.89 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	1-Yr
(in)	(in)	(in)	(in)	(in)	(in)	(in)
3.5	4.5	5.0	6.0	7.0	7.5	2.8

Storm Data Source: Westchester County, NY (NRCS)
Rainfall Distribution Type: Type III
Dimensionless Unit Hydrograph: <standard>

MEG 106100.03 PROP. COND - Larkin Plaza Westchester County, New York

Storm Data

Rainfall Depth by Rainfall Return Period

2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	1-Yr
(in)	(in)	(in)	(in)	(in)	(in)	(in)
3.5	4.5	5.0	6.0	7.0	7.5	2.8

Storm Data Source: Westchester County, NY (NRCS)
Rainfall Distribution Type: Type III
Dimensionless Unit Hydrograph: <standard>

MEG 106100.03 PROP. COND - Larkin Plaza Westchester County, New York

Watershed Peak Table

Sub-Area or Reach Identifier		Flow by 10-Yr (cfs)	25-Yr	Return Period 100-Yr (cfs)	
SUBAREAS Plaza	2.83	4.76	6.08	8.02	
REACHES					
OUTLET	2.83	4.76	6.08	8.02	

MEG 106100.03 PROP. COND - Larkin Plaza Westchester County, New York

Hydrograph Peak/Peak Time Table

Sub-Area	Peak Fi	low and Peak	Time (hr)	by Rainfall	Return Period
or Reach					
Identifier	(cfs)	(cfs)	1cfel	(cfe)	

SUBAREAS
Plaza 2.83 4.76 6.08 8.02
12.24 12.23 12.23 12.23

REACHES

OUTLET 2.83 4.76 6.08 8.02

MEG 106100.03 PROP. COND - Larkin Plaza Westchester County, New York

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	-	Receiving Reach	Sub-Area Description
Plaza	1.89	0.352	84	Outlet	Park Area and River

Total Area: 1.89 (ac)

MEG 106100.03 PROP. COND - Larkin Plaza Westchester County, New York

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimete (ft)	r Velocity (ft/sec)	Travel Time (hr)
Plaza SHEET CHANNEL	99 680	0.0200	0.240			1.500	0.226 0.126
				Ti	me of Con	centration	0.352

MEG 106100.03 PROP. COND - Larkin Plaza Westchester County, New York

Sub-Area Land Use and Curve Number Details

Sub-Area Identifie	•	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
Plaza	Open space; grass cover > 75% (good Paved parking lots, roofs, driveways) C C	1.07 .824	74 98
	Total Area / Weighted Curve Number		1.89	84



TABLE OF FLOWS MEG 106100.03 - LARKIN PLAZA

			Ξ
Project: LARKIN PLAZA	Date	11.26.07	
No: 106100	B.	BERT	
	Chk	Chk SLG	

		Areas	•••••	S	2		Storn	Storm Flow	
Description	Pervious	Impervious	Total			2 Year	10 Year	25 Year	100 Year
	(AC)	(AC)	(AC)		(Hr)	(CFS)	(CFS)	(CFS)	(CES)
i i							, , , , , , , , , , , , , , , , , , , ,	,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
A) Existing Conditions 0.37	0.37	1.52	1.89	94	0.353	3.97	5.90	7.17	9.07
B) Proposed Condition 1.07	1.07	0.82	1.89	84	0.352	2.83	4.76	6.08	8.02
			\$1						
Difference: 0.70	0.70	-0.70	00.0		,,,,,	41.1-	-114	-1 00	

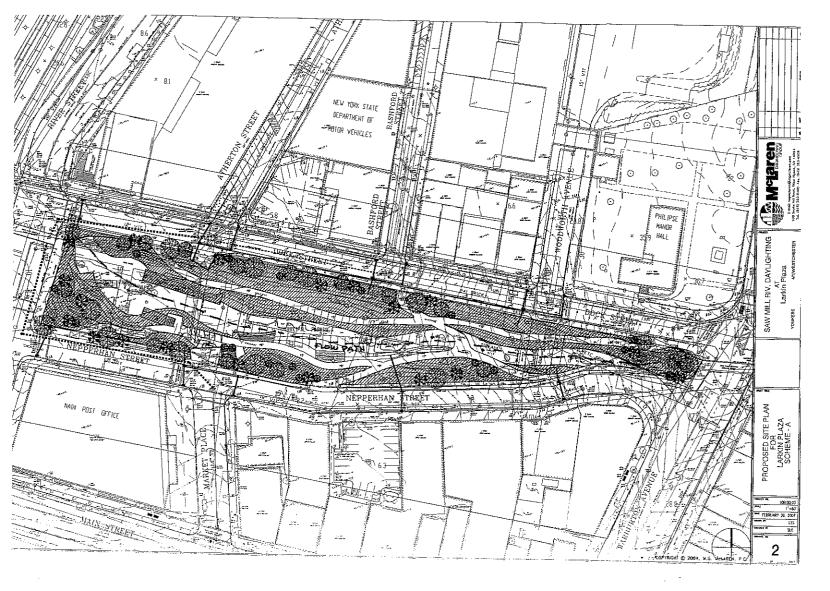




TABLE OF CURVE NUMBER OF DRAINAGE AREAS MEG 106100.03 - LARKIN PLAZA

		Š	SLG			RECOMMEN	IDED CURVE	RECOMMENDED CURVE NUMBER * AREAS (CN*A)	EAS (CN*A)		
			Areas	ω		Pavement and Roads	IMPERVIO	IMPERVIOUS AREA (GRASS COVER)	(SS COVER)	Total CN*A	
Area No	Descriptions	Pervious	Impervious	Total	Total Area	Impervious	Poor	Fair	Good		N O
		(SFT)	(SFT)	(SFT)	(Ac)	866	98	79	74	(C*A)	
A) Existing Conditions	ditions										
ין באופוווופן ססוו	Diantere	Y866									
-	Planters	3533									
	Planters	2415				20 0000		Ì			
:	Planters	1077									
	Planters	1190									
	Planters	152									
	Planters	3428									
	Planters	2147									
	Road and Parking		82524								
	Sub Total	16225	66299	82524	1.89	149.16		29.43		178 58	Po
	1717										
b) Proposed Condition	חמומוסת	1		i							
	Planters	6621									
	raileis	9/69									
	Planters	8896		i							
	Planters	3939									
	Planters	4173							ļ 		
	Planters	: 129									
	Planters	3985									
	Planters	3581									
	Planters	1191									
	Planters	6621									
	Water		3348								
-	Water		2360								
	Water		11652								
	Road and Walkway		18529				İ				
:										İ	
	Sub Total	46635	35889	82524	1,89	80.74			79.22	159.97	84
		-									



PROJECT KIVER	T DRK LENTER	
SHEET NO.	OF	
CALCULATED BY 525	DATE	
CHECKED BY	DATE	
REVISIONS		
SCALE		

AREA	WITIN	YONKERS	To	SMR	~	274.0 AC.	(:	SKE	DAYLIGATING	REPURT)
RIVERPE	saic Stv	dy Dred				28.9BC				

AREDIN YORKERS 245.1 AC.

CN = 91 TC= 17.20 MINUTES

BOW MILL RIVER & PER ACOE BISPORT @ HOUR 12 = 200CFS

Q TOTAL TO SMM CORVER PARK CENTER = 200CFS + Q275AC + SPINER

CRIVER PARC = Q TO SMR ; NOT TO COMBINED SEWER

EXIST. CONDITION

ROYSEC + Spinish = 1323 CFS (SIEH Hyd. 14)

Roma = 200 CFS

PRUP. CONDITION

Q245+ GRNAR = 1304 CRS (SEE Nyclro #20)

1523 CFS

QSMR = 200 CFS 1504 CFS

> QUERRESE = 1523-1504 = 19CFS (1.290 UF FXIST. PERIL)

Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 13

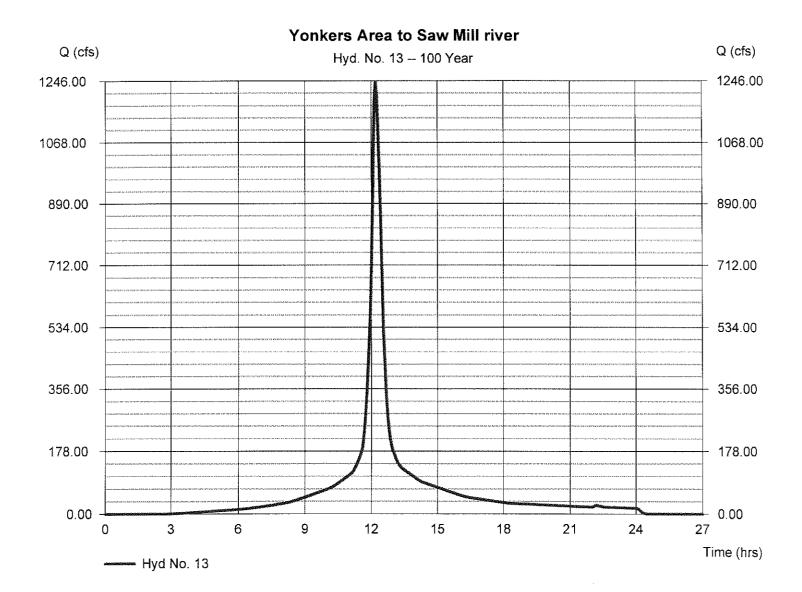
Yonkers Area to Saw Mill river

= SCS Runoff Hydrograph type Storm frequency = 100 yrsTime interval = 3 min Drainage area = 245.100 ac = 0.0 % Basin Slope Tc method = USER = 7.50 inTotal precip. Storm duration = 24 hrs

Peak discharge = 1242.88 cfs Time to peak = 12.20 hrs Hyd. volume = 135.461 acft

Curve number = 91 Hydraulic length = 0 ft

Time of conc. (Tc) = 17.20 min
Distribution = Type III
Shape factor = 484



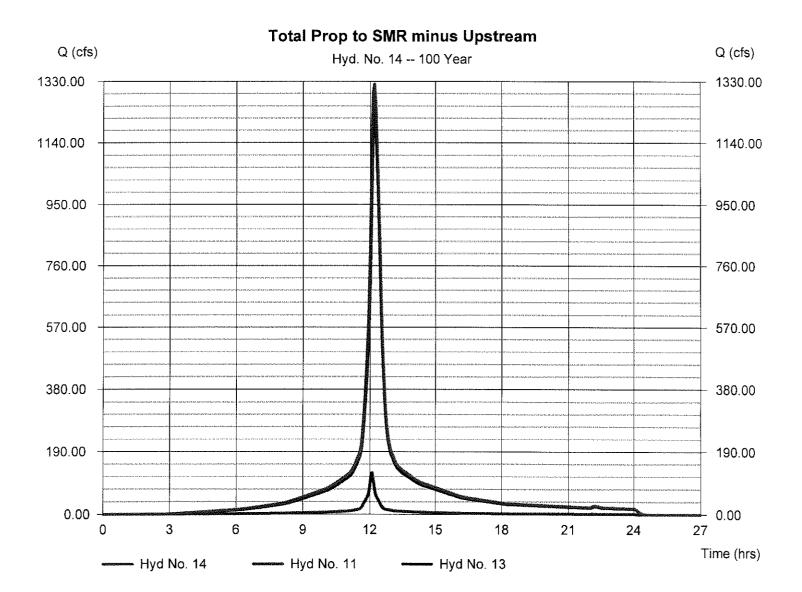
Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

Hyd. No. 14

Total Prop to SMR minus Upstream

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 11, 13 Peak discharge = 1323.16 cfs Time to peak = 12.20 hrs Hyd. volume = 146.275 acft Contrib. drain. areæ 245.100 ac



Hydraflow Hydrographs by Intelisolve v9.01

Tuesday, Dec 4, 2007

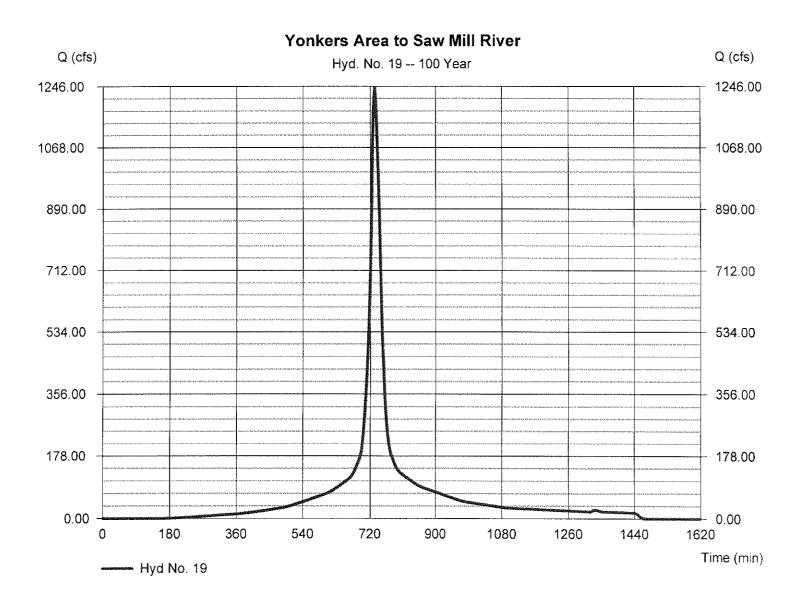
Hyd. No. 19

Yonkers Area to Saw Mill River

Hydrograph type = SCS Runoff Storm frequency = 100 yrs Time interval = 3 min Drainage area = 245.100 ac Basin Slope = 0.0 %

To method = USER Total precip. = 7.50 in Storm duration = 24 hrs Peak discharge = 1242.88 cfs
Time to peak = 732 min
Hyd. volume = 135.461 acft
Curve number = 91

Hydraulic length = 0 ft
Time of conc. (Tc) = 17.20 min
Distribution = Type III
Shape factor = 484



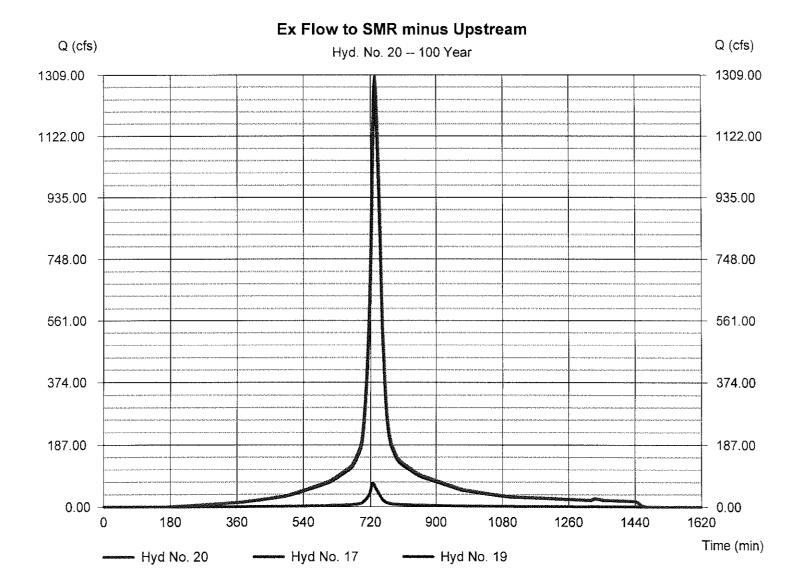
Hydraflow Hydrographs by Intelisoive v9.01

Tuesday, Dec 4, 2007

Hyd. No. 20

Ex Flow to SMR minus Upstream

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 3 min Inflow hyds. = 17, 19 Peak discharge = 1304.42 cfs Time to peak = 732 min Hyd. volume = 142.631 acft Contrib. drain. areæ 245.100 ac





JOB: JOB #: CLIENT: CALC BY: CHK BY:

River Park Center 106100 SFC SLG DATE: 12/5/2007 DATE:

Printed: 12/12/2007

Comparison of River Elevations with Increased flow from Project

Comparison of 100-year Flows vs. 100-year+19 cfs Flows Date: 4-Dec-07

	100-yr Flow (1	540 cfs)	100 yr + 49 c	fs (1589)	Delt	ta	
River Sta.	W.S. Elev.	Chan. Vel.	W.S. Elev.	Chan. Vel.	W.S. Elev.	Chan. Ve	Comment/Notes
10	48.23	13.53	48.26	13.62	0.03	0.09	At Palisades Bridge
100	55.13	6.54	55.22	6.57	0.09	0.03	
217.75	52.78	9.29	52.85	9.33	0.07	0.04	At Mouth of Culvert
238.44	52.68	10	52.75	10.03	0.07	0.03	
278.57	54.97	3.84	55.05	3.84	0.08	0	At Pond Spillway
288	55.11	3.82	55.2	3.82	0.09	0	
374.7	55.13	4.07	55.22	4.08	0.09	0.01	
416.46	55.21	3.68	55.29	3.68	0.08	0	
458.2	55.25	3.46	55.33	3.46	0.08	0	
499.95	55.27	3.49	55.35	3.49	0.08	0	Downstream of Bridg

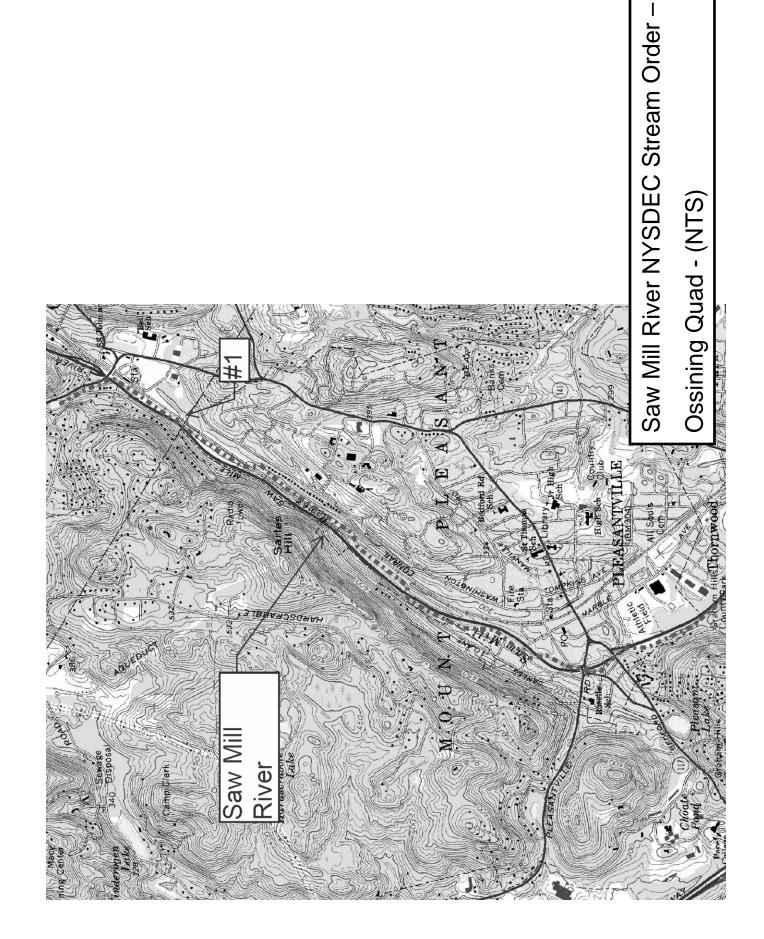
HEC-RAS Plan: prpcongatewa River: SAWMILL RIVER Reach: RIVER PARK Profile: 100 Yr

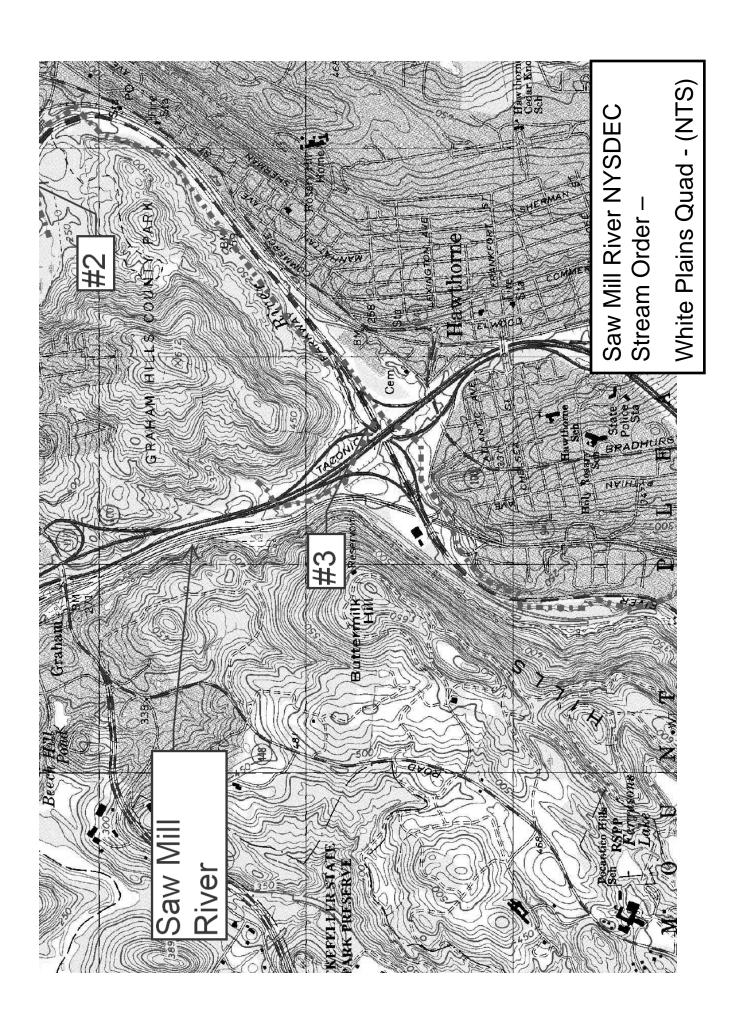
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chrii	Flow Area	Top Width	Froude # Chi
			(cfs)	(ft)	(ft)	(n)	(n)	(ft/ft)	(ft/s)	(It pa)	(ft)	
RIVER PARK	10	100 Yr	1540.00	42.55	48.23	48.23	51.07	0.179820	13.53	113.83	20.07	1,00
RIVER PARK	100	100 Yr	1540.D0	43.40	55.13		55.79	0.024608	6.54	235.54	20.16	0.34
RIVER PARK	190		Culvert									
RIVER PARK	217,75	100 Yr	1540.00	44.52	52.78	50.20	54.12	0.004685	9.29	165.72	20.12	0.57
RIVER PARK	238		Culvert									
RIVER PARK	238,44	100 Yr	1540.00	45.00	52,68	50.69	54.23	0.005726	10.00	154.04	20.12	0.64
RIVER PARK	278		Bridge									
RIVER PARK	278.57	100 Yr	1540.00	45.40	54.97	49.47	55.20	0.000486	3.84	401.34	53.79	0.25
RIVER PARK	285		ini Struct									
RIVER PARK	288	100 Yr	1540.00	45,50	55.11	49.57	55.34	0.000490	3.82	403.18	52.30	0.24
RIVER PARK	374.7	100 Yr	1540.00	46.10	55,13		55.39	0.000595	4.07	378.02	52.05	D.27
RIVER PARK	416,45*	100 Yr	1540.00	46.40	55.21		55.42	0.000486	3.68	418.40	58.87	0.24
RIVER PARK	458.2*	100 Yr	1540.00	46.70	55.25		55.44	0.000403	3,46	445.36	58.83	0.22
RIVER PARK	499.95	100 Yr	1540.00	47.00	55.27		55.45	0.000410	3.49	441.78	56,26	0.22
RIVER PARK	500		Bridge									

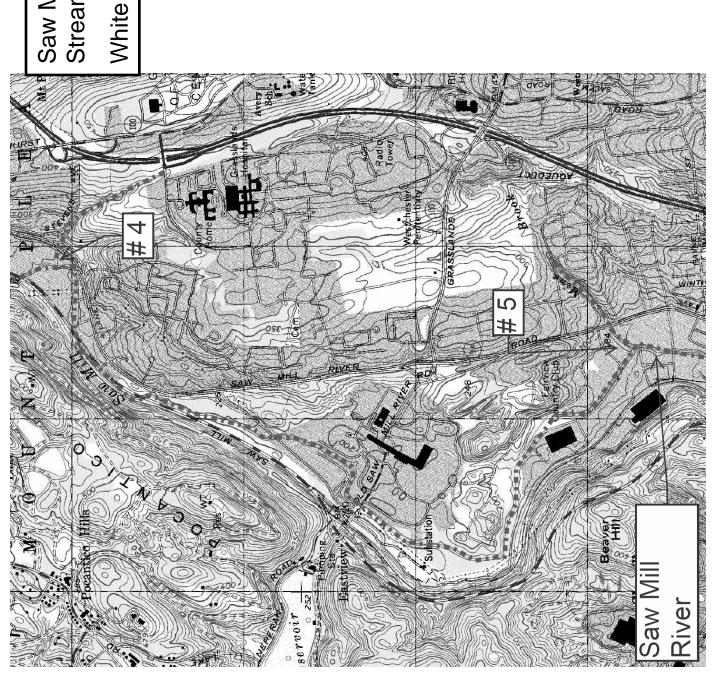
L WITH DESIGN FLOW HEC-RAS Plan: prpcongatewa River: SAWMILL RIVER Reach: RIVER PARK Profile: 100 Yr

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chrif	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ff)	(ft)	(ft/ft)	(fl/s)	(sq ft)	(ft)	
RIVER PARK	10	100 Yr	1559,00	42.55	48.26	48.26	51.14	0.181368	13.62	114.47	20.07	1.01
RIVER PARK	100	100 Yr	1559.00	43.40	55.22		55.89	0.024718	6.57	237.36	20.16	0.34
RIVER PARK	190		Culvert									
RIVER PARK	217,75	100 Yr	1559.00	44.52	52.85	50.23	54.20	0.004690	9.33	167.16	20.12	0.57
RIVER PARK	238		Culvert									
RIVER PARK	238,44	100 Yr	1559.00	45.00	52.75	50.71	54.31	0.005719	10.03	155.49	20.12	0.64
RIVER PARK	278		Bridge									
RIVER PARK	278.57	100 Yr	1559.00	45,40	55.05	49.50	55.28	0.000482	3.84	405.96	54.00	0.25
RIVER PARK	285		Inl Struct									
RIVER PARK	288	100 Yr	1659.00	45.50	55.20	49.61	55.43	0.000487	3.82	407.60	52,47	0.24
RIVER PARK	374.7	100 Yr	1559.00	46.10	55,22		55,47	0,000590	4.08	382.42	52.22	0.27
RIVER PARK	416.45*	100 Yr	1559.00	46.40	55.29		55.50	0.000483	3.68	423,38	59.16	0.24
RIVER PARK	458.2*	100 Yr	1559.00	46.70	55.33		55,52	0.000400	3.46	450.30	58.95	0.22
RIVER PARK	499.95	100 Yr	1559.08	47.00	55,35		55,54	0.000407	3.49	446.49	56.31	0.22
RIVER PARK	500		Bridge	1								

LWITH 19CFS INCRESSE







Saw Mill River NYSDEC Stream Order – White Plains Quad - (NTS)